

Traffic Operations Study

To: **Matt Schumacher**
CAGE Civil Engineering

625 Forest Edge Drive, Vernon Hills, IL 60061
TEL 847.478.9700 ■ FAX 847.478.9701
www.gha-engineers.com

From: Antonio Maravillas, E.I.T.
Transportation Engineer

Bill Grieve, P.E., PTOE
Senior Transportation Engineer

Date: January 9, 2020

Subject: **Field Middle School**
2055 Landwehr Road - Northbrook, IL

Part I. Project Context and Summary Statement

Gewalt Hamilton Associates, Inc. (GHA) has conducted a Traffic Operations Study for the recently repositioned and reconstructed south access drive on Landwehr Road for Field Middle School in Northbrook, Illinois. Formerly, a left-turn storage lane was provided for the previous access drive along Landwehr Road. Prior to the 2019-2020 school year, the access drive was reconstructed further south on Landwehr Road where there is not currently a dedicated left-turn lane.

The following summarizes our findings and provides various recommendations for your consideration. *Exhibits* and *Appendices* referenced are centrally located at the end of this document.

Part II. Background Information

Site Location Map and Roadway Inventory

Exhibit 1 provides a location map of the site, and **Appendix A** provides a photo inventory of the site vicinity. The existing traffic operations in the site area are illustrated on **Exhibit 2**. Pertinent comments to the adjacent roadways include:

Landwehr Road (County Route W80)

- Landwehr Road is a north-south route that is under the jurisdiction of the Cook County Department of Transportation and Highways (CCDOH) and is classified as a Major Collector.
- It provides a two-lane cross section (one through lane in each direction) with sidewalks along both sides of the roadway.
- At its signalized intersection with Techny Road, separate left-turn lanes are provided on both approaches. A pedestrian crosswalk is striped and signed on the northbound approach with 'No Turn on Red' signage posted. Pedestrian countdown signals are also provided on both approaches.
- South of Techny Road, a three-lane cross section is present with a dedicated left-turn lane for the former school entrance point. It returns to a two-lane cross section immediately north of the relocated entrance.

Landwehr Road School Access

- Two access points are provided for the school along Landwehr Road with the northern drive serving as exit only and the southern drive serving as enter only. 'Do Not Enter' signage is posted at the northern drive with exiting traffic having stop control.
- At the northern drive, separate left and right-turn lanes are provided.
- Crosswalks are striped at both access points.

Techny Road (County Route A76)

- Techny Road is an east-west route that is also under the jurisdiction of CCDOTH and is classified as a Major Collector.
- It provides a two-lane cross section (one through lane in each direction) with sidewalks along both sides of the roadway west of Landwehr Road. East of Landwehr Road, a sidewalk is provided along the northern side of the roadway.
- At its signalized intersection with Landwehr Road, separate left-turn lanes are provided on both approaches. Pedestrian crosswalks are signed with 'No Turn on Red' signage posted on both approaches. Pedestrian countdown signals are also provided on both approaches and a crosswalk is striped on the westbound approach.

Techny Road School Access

- Two access points are provided for the school along Techny Road. The western drive provides one inbound and one outbound lane while the eastern drive provides two outbound lanes. 'Do Not Enter' signage is posted at the eastern drive with exiting traffic having stop control.

Existing Traffic

Peak period traffic turning movement counts were conducted by GHA on Tuesday, November 19, 2019 from 7:00 to 9:00 AM and 2:30 to 6:00 PM at the Landwehr Road and Techny Road intersection, as well as at all four school access driveways. The observed weekday morning peak hour occurred from 7:30-8:30 AM, which coincided with the school's morning drop-off period. The observed weekday evening peak hour for the school occurred from 3:30-4:30 PM, coinciding with the schools 3:30 dismissal. The observed weekday evening street peak hour occurred from 5:00-6:00 PM on Landwehr Road and Techny Road. Summaries of the existing traffic counts can be found in ***Appendix B***.

Exhibit 3 summarizes the existing weekday morning and evening peak period traffic and pedestrian volumes. ***Exhibit 3*** also provides the AADT, or 24-hour volume, along Landwehr Road and Techny Road, as published by the Illinois Department of Transportation (IDOT) on their website www.gettingaroundillinois.com.

Crash Analysis

Observing the most recent available crash history can determine if any roadway improvements are needed to improve safety along the surrounding roadways. Crash data from 2014-2018 was obtained from the IDOT Bureau of Data Collection for Landwehr Road and Techny Road in the site vicinity. ***Appendix C*** summarizes the 5-year (2014-2018) crash history at the Landwehr Road intersection with Techny Road. As can be seen, 22 crashes occurred during the 5-year study period, of which 16 (73%) were rear ends.

Only two (2) crashes occurred on Techny Road north of the school while zero (0) crashes occurred on Landwehr Road west of the school during the 5-year study period. The only crash pattern that occurred in the site area were the rear end collisions at the intersection of Landwehr Road and Techny Road, which is typical for a signalized intersection. Thus, no roadway improvements are necessary based on crash history.

Part III. Traffic Evaluation

Turn Lane Analysis

With the recent relocation of the Landwehr Road Access drive, there is no dedicated left-turn lane into the school on Landwehr Road. This study examined whether a left-turn storage lane should be considered for the new access driveway.

The Illinois Department of Transportation's *Bureau of Design and Environment Manual* was used to determine the need for auxiliary lanes. A left-turn lane is warranted at any unsignalized intersection that satisfies the criterion depicted in Figure 36-3.G for two-lane highways with a speed limit of 40 mph or less. The chart is included on ***Exhibit 4 – Part A***. Based on the existing traffic volumes, an auxiliary left-turn lane is warranted on southbound Landwehr Road at the south access drive (see ***Exhibit 4 – Part B***).

Discussion Point. The left-turn volumes along Landwehr Road at the south school access drive account for a low percentage of total southbound traffic (3% during the morning peak hour and 1% during the evening school peak hour). Based on the low left-turn volumes, posted speed limit, and lack of recent crash history, it is recommended that a southbound left-turn lane not be initially constructed at the new access drive, but that traffic conditions should be monitored to determine if providing a southbound left turn lane would improve operational safety.

Intersection Capacity Analyses

Intersection capacity analyses were conducted using the Highway Capacity Software (HCS) and results are shown in ***Exhibit 5***. The analysis parameters are listed in Part A, as published in the Transportation Research Board's (TRB) *Highway Capacity Manual – 6th Edition*, 2016 (HCM). At signalized intersections, Level of Service (LOS) "reports" traffic operations using the letter designations "A" (best) through "F" (worst). LOS reports operations based on the average control delay per vehicle in seconds.

LOS C is often referred to as the intersection "design" guideline and LOS D is usually considered as providing the lower threshold of "acceptable" operations. LOS E and F are usually considered "unacceptable". At unsignalized intersections, the HCS measurement is approach delay in seconds.

Capacity analyses were conducted at the Landwehr Road and Techny Road intersection, as well as at all four school access drives. The results are summarized in ***Exhibit 5***. The HCS summary printouts are provided in ***Appendix D***.

Key Finding. The capacity analyses results indicate that all movements at every intersection tested currently operate at or better than the acceptable LOS D. The intersection of Landwehr Road and Techny Road operates at overall LOS C during all three peak periods.

Key Finding. The capacity analyses results indicate that northbound queues at the Landwehr Road and Techny Road intersection exceed 500 feet during the morning peak hour. Field observations showed back-ups as far as the southern school access drive.

Part IV. Conclusions and Recommendations

A traffic analysis was performed for Field Middle School in Northbrook, Illinois. The capacity analysis results indicate that traffic along Landwehr Road and Techny Road operate at acceptable levels of service during the morning and both evening peak hours. The following provides recommendations to any roadway improvements for your consideration:

- Along Landwehr Road, a southbound left-turn lane is warranted at the south access drive based on IDOT's criteria. Field observations also showed vehicles frequently bypassing on the shoulder when another vehicle was waiting to turn left. However, as previously discussed, it is not recommended that a left-turn lane be constructed. Instead, due to the low left turn volumes, turning restriction signage should be posted at the drive for the morning hours when northbound traffic is heaviest, and back-ups occur from the Techny Road signal.
- 'Do Not Pass On Shoulder' signage should be posted for southbound traffic at the new access drive.
- Yellow diagonal crosshatch markings should be striped on the former turn lane along Landwehr Road.

Part V. Technical Addendum

The following Appendices were previously referenced. They provide technical support for our observations, findings and recommendations discussed in the text.

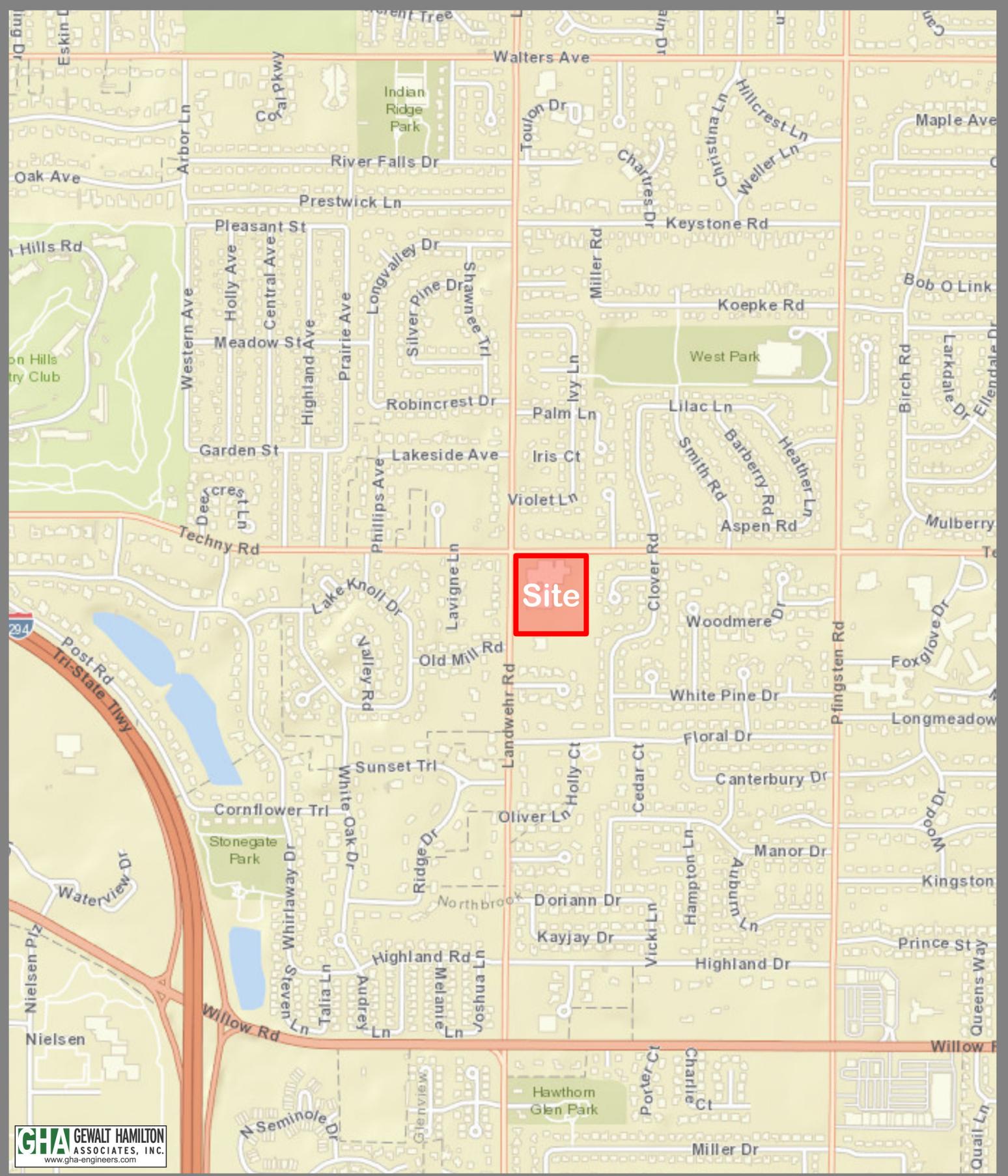
Exhibits

1. Site Location Map
2. Existing Traffic Operations
3. Existing Traffic and Pedestrians
4. Turn Lane Requirements
5. Intersection Capacity Analyses

Appendices

- A. Photo Inventory
- B. Traffic Count Summary Sheets
- C. Crash Summary
- D. Capacity Analysis Worksheets

EXHIBITS



GHA GEWALT HAMILTON
ASSOCIATES, INC.
www.gha-engineers.com

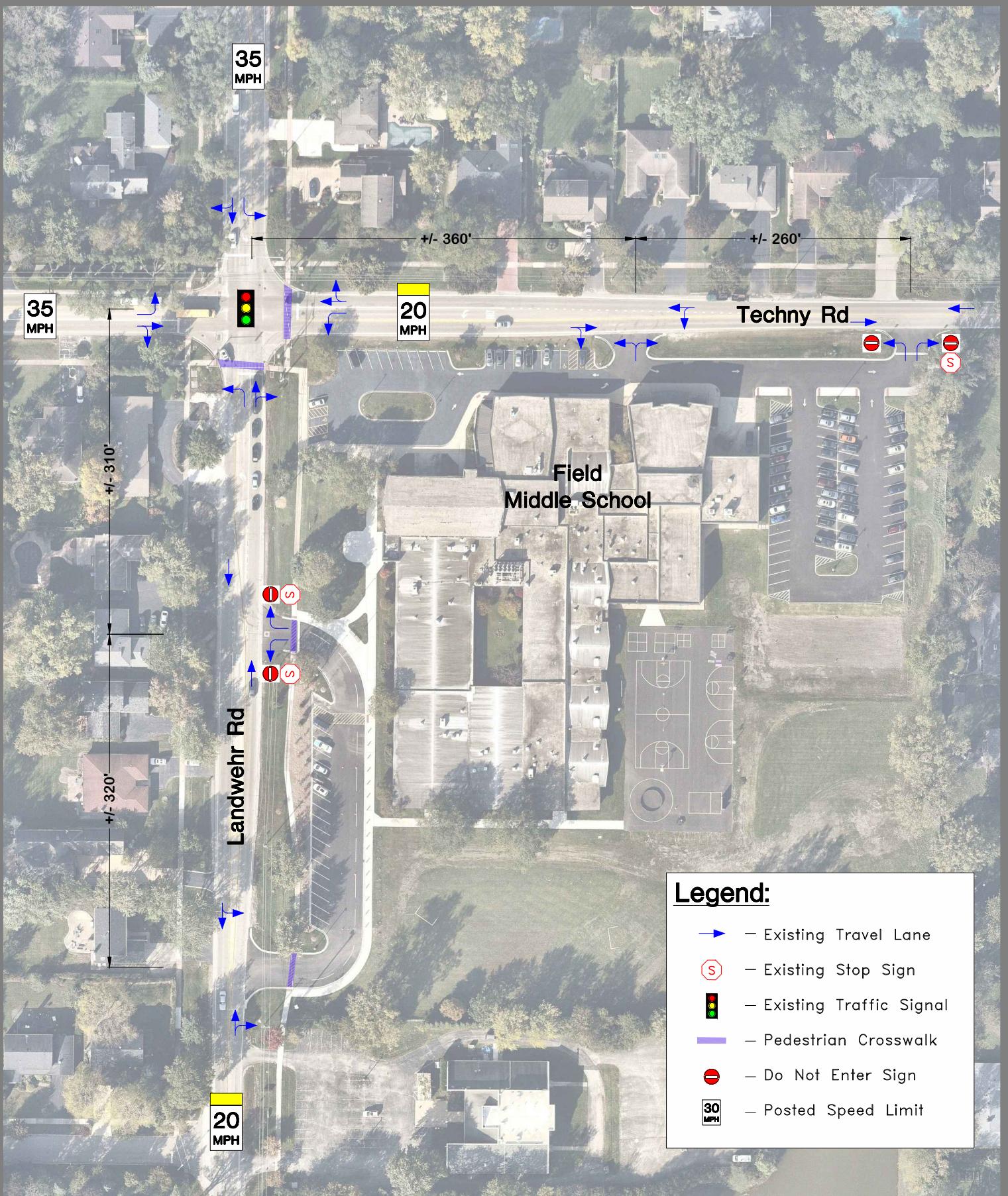


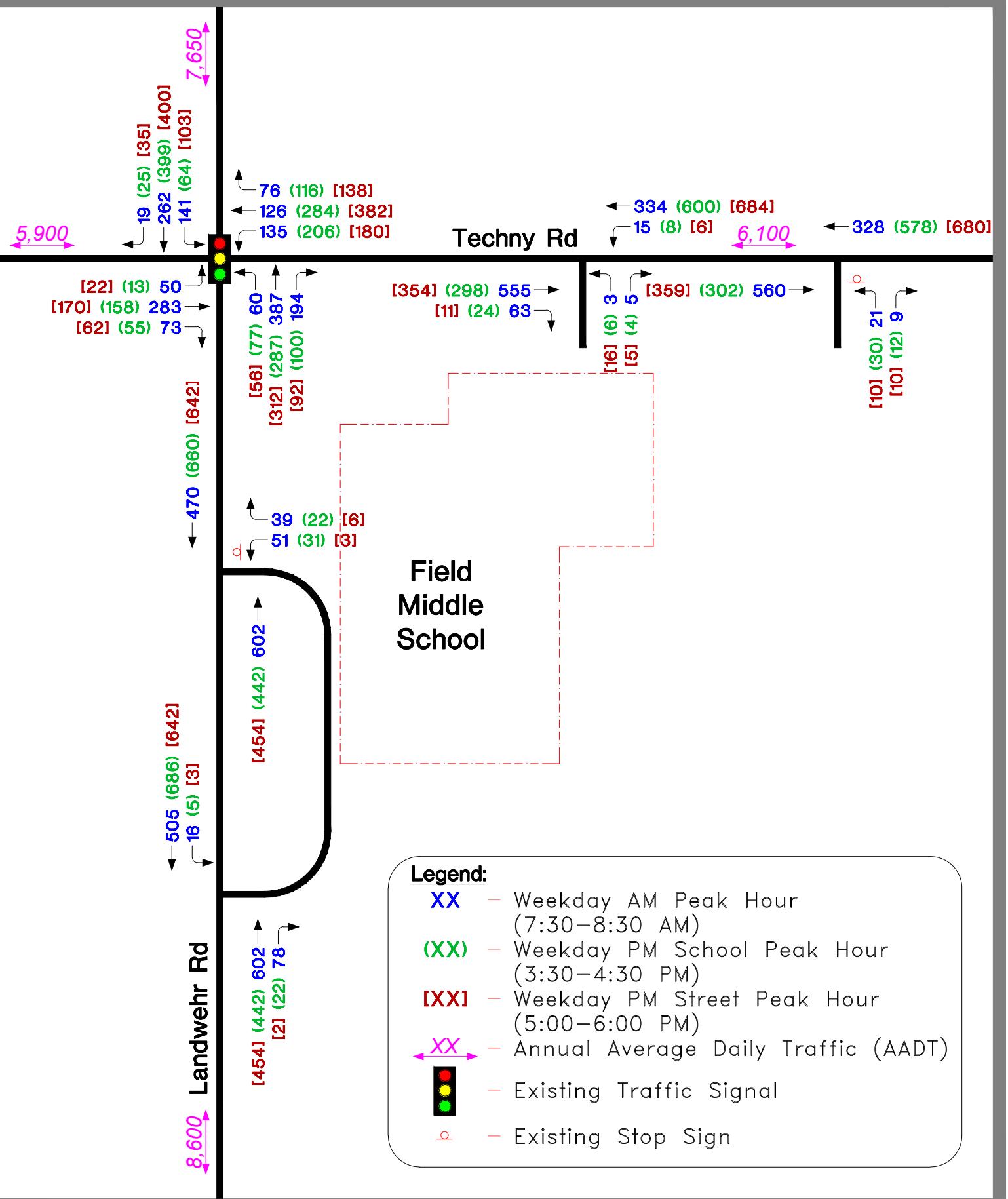
1 inch = 1,000
Feet

Map Center: 87.85884°W 42.11457°N

Exhibit 1 - Location Map

Field Middle School
2055 Landwehr Road, Northbrook, IL





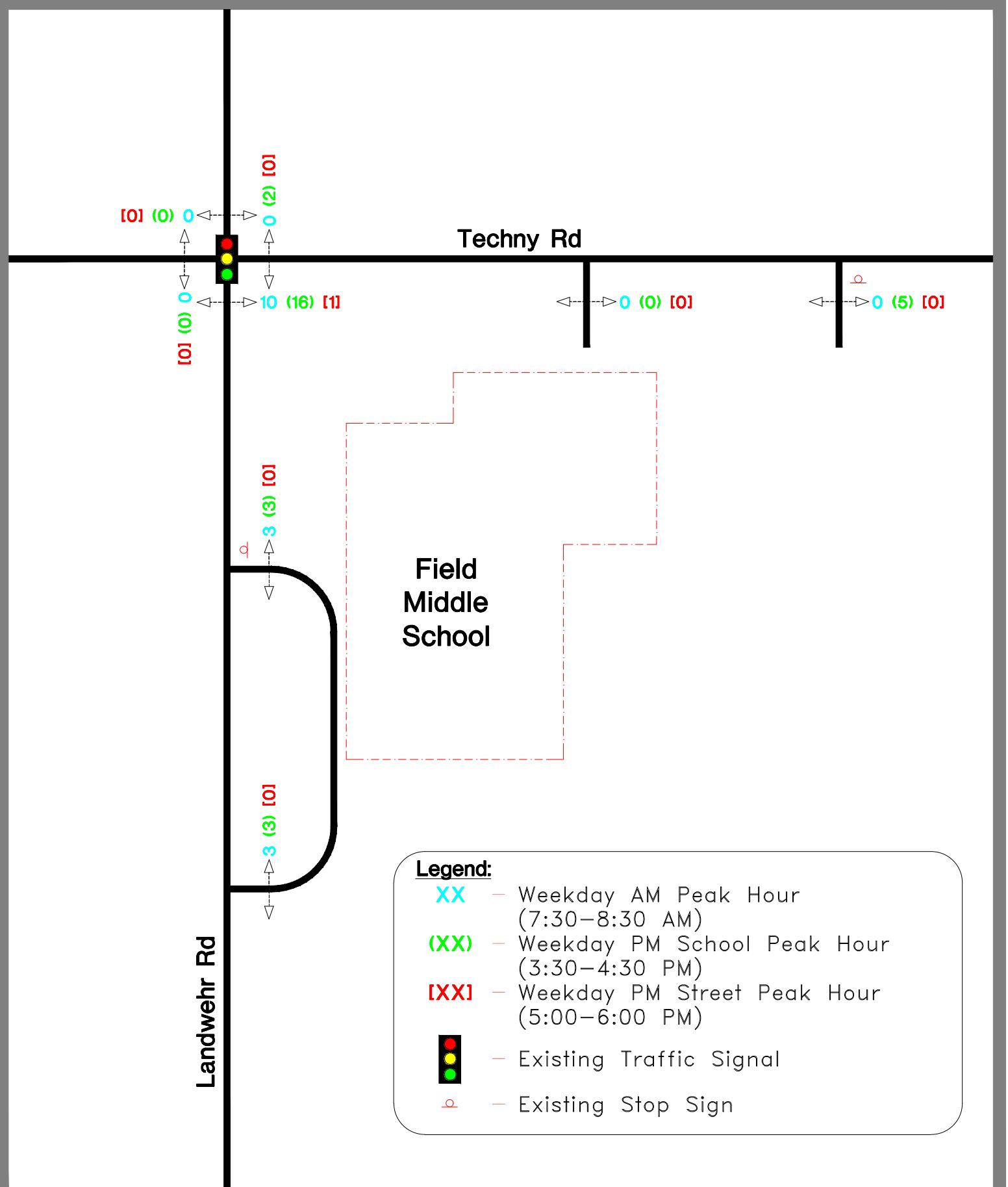
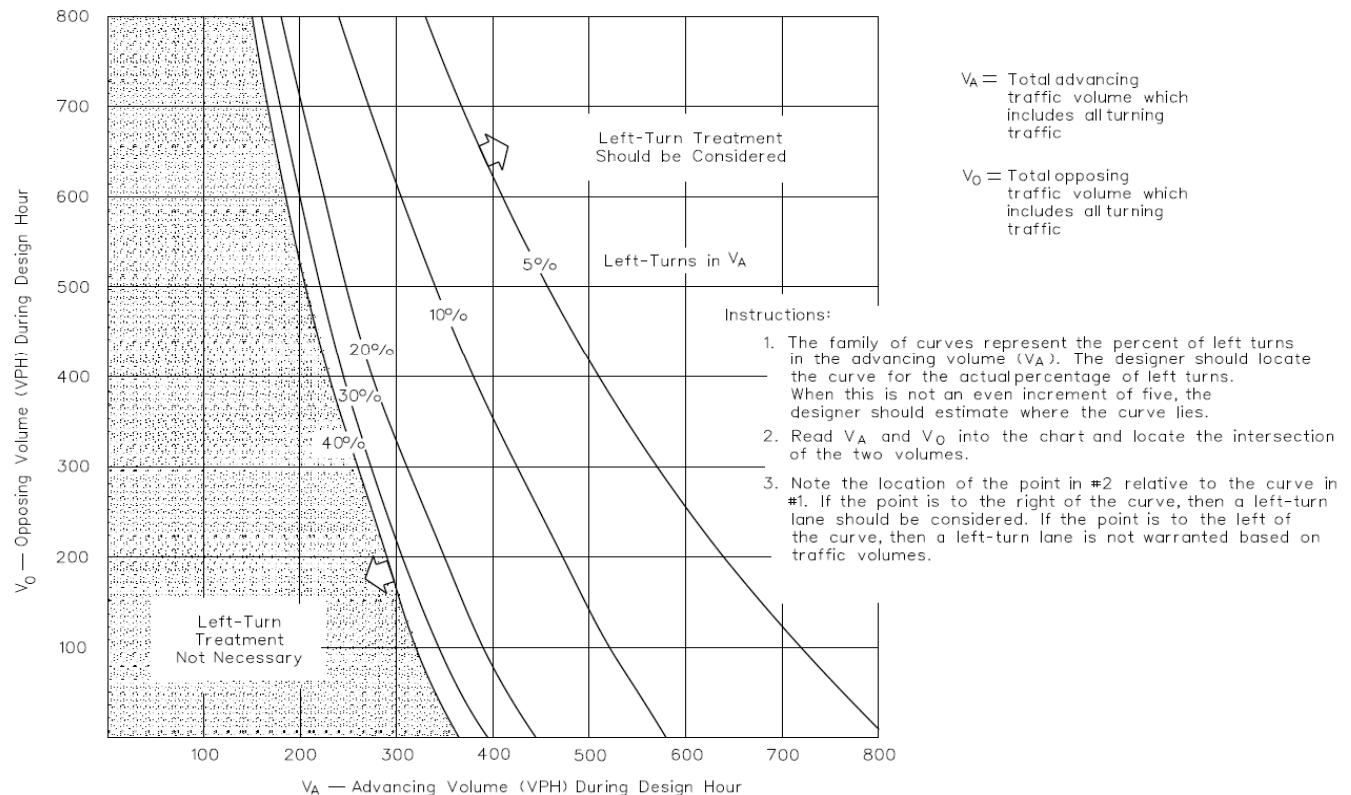


Exhibit 4
Turn Lane Requirements
Field Middle School - Northbrook, Illinois

Part A. Parameters - Minimum Volume Required for Warranting Exclusive Left-Turn Lane
(Source: IDOT Bureau of Design and Environment Manual)



Part B. Results

Left-Turn Lane Warrant					
1. Landwehr Rd at School Access	NB Approach Total	SB Approach Total	SB Left Turns	Satisfied?	
A. Weekday Morning Peak Hour Existing Traffic (See Exhibit 3)	680	521	16	3%	YES
B. Weekday Evening Peak Hour Existing Traffic (See Exhibit 3)	464	691	5	1%	YES

Exhibit 5
Intersection Capacity and Queue Analyses

Field Middle School - Northbrook, IL

Part A. Parameters - Type of Traffic Control (source: Highway Capacity Manual 6th Edition)

I. Traffic Signals

<u>LOS</u>	<u>Delay (sec / veh)</u>	<u>Description</u>
A	<10	All signal phases clear waiting vehicles without delay
B	>10 and < 20	Minimal delay experienced on select signal phases
C	>20 and < 35	Some delay experienced on several phases; often used as design criteria
D	>35 and < 55	Usually considered as the acceptable delay standard
E	>55 and < 80	Very long delays experienced during the peak hours
F	>80	Unacceptable delays experienced throughout the peak hours

II. Stop Sign

<u>LOS</u>	<u>Delay (sec / veh)</u>
A	< 10
B	>10 and < 15
C	>15 and < 25
D	>25 and < 35
E	>35 and < 50
F	>50

Part B. Results

Roadway Conditions	LOS Per Movement By Approach												Intersection / Approach		
	> = Shared Lane			- = Non Critical or not Allowed Movement											
	Eastbound			Westbound			Northbound			Southbound			Delay (sec / veh)	LOS	
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT			
1. Landwehr Rd at Techny Rd	Signalized	Eastbound			Westbound			Northbound			Southbound			Intersection Delay	
A. Weekday Morning Peak Hour Existing Traffic (See Exhibit 3)	<ul style="list-style-type: none"> • Current • 95th Queue Length (ft) 			C D < 41 351 -	C C < 107 188 -	B D < 38 539 -	C B < 88 205 -	C 31.0	C						
B. Weekday Evening School Peak Hour Existing Traffic (See Exhibit 3)	<ul style="list-style-type: none"> • Current • 95th Queue Length (ft) 			C C < 7 139 -	B C < 98 235 -	B C < 33 216 -	B C < 28 245 -	C 20.7	C						
C. Weekday Evening Street Peak Hour Existing Traffic (See Exhibit 3)	<ul style="list-style-type: none"> • Current • 95th Queue Length (ft) 			C C < 13 162 -	B C < 95 349 -	B C < 32 280 -	B C < 56 294 -	C 24.1	C						
2. Landwehr Rd at North School Access	TWSC - WB Stop	Eastbound			Westbound			Northbound			Southbound			WB Approach Delay	
A. Weekday Morning Peak Hour Existing Traffic (See Exhibit 3)	<ul style="list-style-type: none"> • Current • 95th Queue Length (ft) 			D B A - 28 8 0 -	A - - 0	A - - 0	A - - 0	22.7	C						
B. Weekday Evening School Peak Hour Existing Traffic (See Exhibit 3)	<ul style="list-style-type: none"> • Current • 95th Queue Length (ft) 			D B A - 15 3 0 -	A - - 0	A - - 0	A - - 0	19.9	C						
C. Weekday Evening Street Peak Hour Existing Traffic (See Exhibit 3)	<ul style="list-style-type: none"> • Current • 95th Queue Length (ft) 			C B A - 0 0 0 -	A - - 0	A - - 0	A - - 0	15.0	C						
3. Landwehr Rd at South School Access	TWSC - WB Stop	Eastbound			Westbound			Northbound			Southbound			SB Approach Delay	
A. Weekday Morning Peak Hour Existing Traffic (See Exhibit 3)	<ul style="list-style-type: none"> • Current • 95th Queue Length (ft) 			- - A < > A - - 0 - - 3	A < > A 0 - - 0	A < > A 0 - - 0	A < > A 0 - - 0	0.6	A						
B. Weekday Evening School Peak Hour Existing Traffic (See Exhibit 3)	<ul style="list-style-type: none"> • Current • 95th Queue Length (ft) 			- - A < > A - - 0 - - 0	A < > A 0 - - 0	A < > A 0 - - 0	A < > A 0 - - 0	0.1	A						
C. Weekday Evening Street Peak Hour Existing Traffic (See Exhibit 3)	<ul style="list-style-type: none"> • Current • 95th Queue Length (ft) 			- - A < > A - - 0 - - 0	A < > A 0 - - 0	A < > A 0 - - 0	A < > A 0 - - 0	0.1	A						

Part B. Results

Roadway Conditions	LOS Per Movement By Approach												Intersection / Approach																
	Eastbound			Westbound			Northbound			Southbound																			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	Delay (sec / veh)	LOS															
4. Techny Rd at West School Access	TWSC - NB Stop	Eastbound			Westbound			Northbound			Southbound			NB Approach Delay															
A. Weekday Morning Peak Hour		<ul style="list-style-type: none"> • Current • 95th Queue Length (ft) 			<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td style="text-align: center;">A</td> <td style="text-align: center;"><</td> <td style="text-align: center;">></td> <td style="text-align: center;">A</td> </tr> <tr> <td style="text-align: center;">0</td> <td style="text-align: center;">-</td> <td style="text-align: center;">-</td> <td style="text-align: center;">3</td> </tr> </table>			A	<	>	A	0	-	-	3	<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td style="text-align: center;">C</td> <td style="text-align: center;"><</td> <td></td> </tr> <tr> <td style="text-align: center;">3</td> <td style="text-align: center;">-</td> <td></td> </tr> </table>			C	<		3	-					15.2	C
A	<	>	A																										
0	-	-	3																										
C	<																												
3	-																												
Existing Traffic (See Exhibit 3)														-	-														
B. Weekday Evening School Peak Hour		<ul style="list-style-type: none"> • Current • 95th Queue Length (ft) 			<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td style="text-align: center;">A</td> <td style="text-align: center;"><</td> <td style="text-align: center;">></td> <td style="text-align: center;">A</td> </tr> <tr> <td style="text-align: center;">0</td> <td style="text-align: center;">-</td> <td style="text-align: center;">-</td> <td style="text-align: center;">0</td> </tr> </table>			A	<	>	A	0	-	-	0	<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td style="text-align: center;">B</td> <td style="text-align: center;"><</td> <td></td> </tr> <tr> <td style="text-align: center;">3</td> <td style="text-align: center;">-</td> <td></td> </tr> </table>			B	<		3	-					14.8	B
A	<	>	A																										
0	-	-	0																										
B	<																												
3	-																												
Existing Traffic (See Exhibit 3)														-	-														
C. Weekday Evening Street Peak Hour		<ul style="list-style-type: none"> • Current • 95th Queue Length (ft) 			<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td style="text-align: center;">A</td> <td style="text-align: center;"><</td> <td style="text-align: center;">></td> <td style="text-align: center;">A</td> </tr> <tr> <td style="text-align: center;">0</td> <td style="text-align: center;">-</td> <td style="text-align: center;">-</td> <td style="text-align: center;">0</td> </tr> </table>			A	<	>	A	0	-	-	0	<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td style="text-align: center;">C</td> <td style="text-align: center;"><</td> <td></td> </tr> <tr> <td style="text-align: center;">5</td> <td style="text-align: center;">-</td> <td></td> </tr> </table>			C	<		5	-					18.8	C
A	<	>	A																										
0	-	-	0																										
C	<																												
5	-																												
Existing Traffic (See Exhibit 3)														-	-														
5. Techny Rd at East School Access	TWSC - NB Stop	Eastbound			Westbound			Northbound			Southbound			NB Approach Delay															
A. Weekday Morning Peak Hour		<ul style="list-style-type: none"> • Current • 95th Queue Length (ft) 			<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td style="text-align: center;">A</td> <td style="text-align: center;">-</td> <td style="text-align: center;">-</td> <td style="text-align: center;">A</td> </tr> <tr> <td style="text-align: center;">0</td> <td style="text-align: center;">-</td> <td style="text-align: center;">-</td> <td style="text-align: center;">0</td> </tr> </table>			A	-	-	A	0	-	-	0	<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td style="text-align: center;">C</td> <td style="text-align: center;">B</td> <td></td> </tr> <tr> <td style="text-align: center;">8</td> <td style="text-align: center;">3</td> <td></td> </tr> </table>			C	B		8	3					16.6	C
A	-	-	A																										
0	-	-	0																										
C	B																												
8	3																												
Existing Traffic (See Exhibit 3)														-	-														
B. Weekday Evening School Peak Hour		<ul style="list-style-type: none"> • Current • 95th Queue Length (ft) 			<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td style="text-align: center;">A</td> <td style="text-align: center;">-</td> <td style="text-align: center;">-</td> <td style="text-align: center;">A</td> </tr> <tr> <td style="text-align: center;">0</td> <td style="text-align: center;">-</td> <td style="text-align: center;">-</td> <td style="text-align: center;">0</td> </tr> </table>			A	-	-	A	0	-	-	0	<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td style="text-align: center;">C</td> <td style="text-align: center;">B</td> <td></td> </tr> <tr> <td style="text-align: center;">10</td> <td style="text-align: center;">3</td> <td></td> </tr> </table>			C	B		10	3					16.1	C
A	-	-	A																										
0	-	-	0																										
C	B																												
10	3																												
Existing Traffic (See Exhibit 3)														-	-														
C. Weekday Evening Street Peak Hour		<ul style="list-style-type: none"> • Current • 95th Queue Length (ft) 			<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td style="text-align: center;">A</td> <td style="text-align: center;">-</td> <td style="text-align: center;">-</td> <td style="text-align: center;">A</td> </tr> <tr> <td style="text-align: center;">0</td> <td style="text-align: center;">-</td> <td style="text-align: center;">-</td> <td style="text-align: center;">0</td> </tr> </table>			A	-	-	A	0	-	-	0	<table border="1" style="width: 100%; border-collapse: collapse;"> <tr> <td style="text-align: center;">C</td> <td style="text-align: center;">B</td> <td></td> </tr> <tr> <td style="text-align: center;">3</td> <td style="text-align: center;">0</td> <td></td> </tr> </table>			C	B		3	0					15.6	C
A	-	-	A																										
0	-	-	0																										
C	B																												
3	0																												
Existing Traffic (See Exhibit 3)														-	-														

APPENDIX A

Photo Inventory



Northbound Landwehr Rd
(Techny Rd intersection)



Southbound Landwehr Rd
(Techny Rd intersection)



Eastbound Techny Rd
(Landwehr Rd intersection)



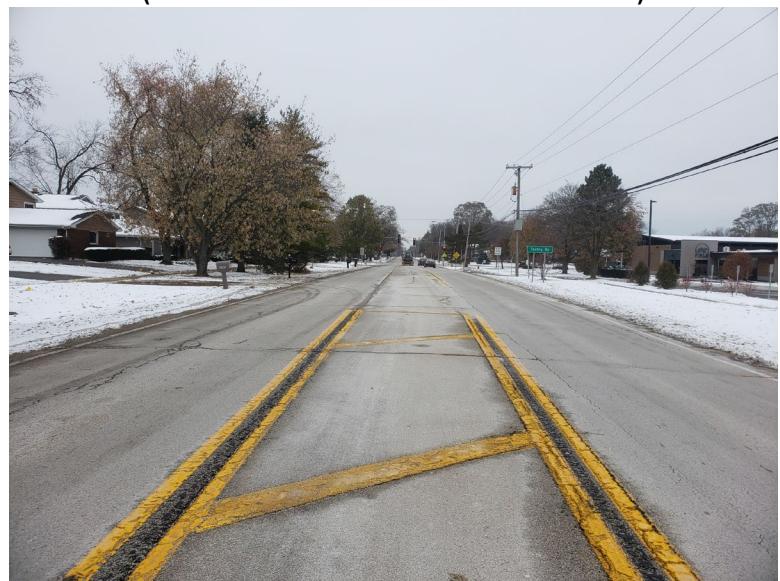
Westbound Techny Rd
(Landwehr Rd intersection)



Northbound Landwehr Rd
(South school access drive intersection)



Southbound Landwehr Rd
(North school access drive intersection)



Northbound Landwehr Rd
(between school access drives)



Southbound Landwehr Rd
(between school access drives)



North school access drive
(from Landwehr Rd, facing East)



South school access drive
(from Landwehr Rd, facing East)



School access drive
(off Landwehr Rd, facing North)



North school access drive
(at Landwehr Rd, facing West)



Eastbound Techny Rd
(West school access drive intersection)



Westbound Techny Rd
(East school access drive intersection)



Eastbound Techny Rd
(between school access drives)



Westbound Techny Rd
(between school access drives)

APPENDIX B

Traffic Count Summary Sheets

Gewalt Hamilton Associates Inc.
625 Forest Edge Drive

Vernon Hills, Illinois, United States 60061
(847) 478-9700 poster@gha-engineers.com

Count Name: Techny Rd @ Landwehr Rd
Site Code:
Start Date: 11/19/2019
Page No: 1

Turning Movement Data

Start Time	Landwehr Southbound						Techny Westbound						Landwehr Northbound						Techny Eastbound						Int. Total
	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	
7:00 AM	2	50	21	0	0	73	14	15	28	0	0	57	24	49	3	0	0	76	7	46	0	0	0	53	259
7:15 AM	6	65	25	0	0	96	14	20	38	0	0	72	50	55	4	0	0	109	11	79	3	0	0	93	370
7:30 AM	5	76	55	0	0	136	15	29	31	0	0	75	50	67	5	0	0	122	18	86	15	0	0	119	452
7:45 AM	5	61	40	0	0	106	29	26	34	0	0	89	53	112	10	0	3	175	24	74	18	0	0	116	486
Hourly Total	18	252	141	0	0	411	72	90	131	0	0	293	177	283	22	0	3	482	60	285	36	0	0	381	1567
8:00 AM	5	62	23	0	0	90	21	40	34	0	0	95	46	111	29	0	3	186	15	60	12	0	0	87	458
8:15 AM	4	63	23	0	0	90	11	31	36	0	0	78	45	97	16	0	4	158	16	57	5	0	0	78	404
8:30 AM	6	55	14	0	0	75	16	41	42	0	0	99	29	80	9	0	1	118	12	62	4	0	0	78	370
8:45 AM	2	59	12	0	0	73	12	32	37	0	2	81	39	115	4	0	0	158	11	45	7	0	0	63	375
Hourly Total	17	239	72	0	0	328	60	144	149	0	2	353	159	403	58	0	8	620	54	224	28	0	0	306	1607
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
2:30 PM	3	63	13	0	0	79	18	50	24	0	0	92	8	54	5	0	0	67	7	38	3	0	0	48	286
2:45 PM	2	51	18	0	0	71	18	52	15	0	0	85	26	68	18	0	0	112	8	30	1	0	0	39	307
Hourly Total	5	114	31	0	0	150	36	102	39	0	0	177	34	122	23	0	0	179	15	68	4	0	0	87	593
3:00 PM	5	75	10	0	0	90	24	48	26	0	0	98	13	62	14	0	0	89	4	36	8	0	0	48	325
3:15 PM	7	67	24	0	0	98	32	67	36	0	0	135	19	57	7	0	0	83	15	36	8	0	0	59	375
3:30 PM	8	94	9	0	0	111	37	73	46	0	0	156	31	77	30	0	12	138	7	44	5	0	0	56	461
3:45 PM	7	92	22	0	0	121	32	65	54	0	1	151	27	66	16	0	0	109	6	28	3	0	0	37	418
Hourly Total	27	328	65	0	0	420	125	253	162	0	1	540	90	262	67	0	12	419	32	144	24	0	0	200	1579
4:00 PM	4	94	18	0	0	116	27	64	54	0	1	145	20	65	13	0	4	98	24	40	2	0	0	66	425
4:15 PM	6	119	15	0	0	140	20	82	52	0	0	154	22	79	18	0	0	119	18	40	3	0	0	61	474
4:30 PM	11	81	19	0	0	111	18	69	49	0	0	136	33	75	11	0	0	119	15	35	5	0	0	55	421
4:45 PM	4	106	24	0	0	134	27	68	68	0	0	163	20	64	9	0	0	93	14	41	7	0	0	62	452
Hourly Total	25	400	76	0	0	501	92	283	223	0	1	598	95	283	51	0	4	429	71	156	17	0	0	244	1772
5:00 PM	10	103	24	0	0	137	28	79	40	0	0	147	28	80	11	0	0	119	14	51	3	0	0	68	471
5:15 PM	9	95	26	0	0	130	36	96	48	0	0	180	22	80	13	0	0	115	10	42	8	0	0	60	485
5:30 PM	8	110	29	0	0	147	27	118	38	0	0	183	21	74	17	0	0	112	18	44	6	0	0	68	510
5:45 PM	8	92	24	0	0	124	47	89	54	0	0	190	21	78	15	0	1	114	20	32	5	0	0	57	485
Hourly Total	35	400	103	0	0	538	138	382	180	0	0	700	92	312	56	0	1	460	62	169	22	0	0	253	1951
Grand Total	127	1733	488	0	0	2348	523	1254	884	0	4	2661	647	1665	277	0	28	2589	294	1046	131	0	0	1471	9069
Approach %	5.4	73.8	20.8	0.0	-	-	19.7	47.1	33.2	0.0	-	-	25.0	64.3	10.7	0.0	-	-	20.0	71.1	8.9	0.0	-	-	-
Total %	1.4	19.1	5.4	0.0	-	25.9	5.8	13.8	9.7	0.0	-	29.3	7.1	18.4	3.1	0.0	-	28.5	3.2	11.5	1.4	0.0	-	16.2	-
Lights	121	1682	471	0	-	2274	507	1227	868	0	-	2602	627	1621	269	0	-	2517	289	1018	128	0	-	1435	8828
% Lights	95.3	97.1	96.5	-	-	96.8	96.9	97.8	98.2	-	-	97.8	96.9	97.4	97.1	-	-	97.2	98.3	97.3	97.7	-	-	97.6	97.3
Mediums	6	37	15	0	-	58	12	22	14	0	-	48	17	38	8	0	-	63	4	24	3	0	-	31	200
% Mediums	4.7	2.1	3.1	-	-	2.5	2.3	1.8	1.6	-	-	1.8	2.6	2.3	2.9	-	-	2.4	1.4	2.3	2.3	-	-	2.1	2.2
Articulated Trucks	0	14	2	0	-	16	4	5	2	0	-	11	3	6	0	0	-	9	1	4	0	0	-	5	41
% Articulated Trucks	0.0	0.8	0.4	-	-	0.7	0.8	0.4	0.2	-	-	0.4	0.5	0.4	0.0	-	-	0.3	0.3	0.4	0.0	-	-	0.3	0.5

Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	0.0	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	-	0	-	-	-	-	4	-	-	-	-	28	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	100.0	-	-	-	-	100.0	-	-

Gewalt Hamilton Associates Inc.
625 Forest Edge Drive

Vernon Hills, Illinois, United States 60061
(847) 478-9700 poster@gha-engineers.com

Count Name: Techny Rd @ Landwehr Rd
Site Code:
Start Date: 11/19/2019
Page No: 4

Turning Movement Peak Hour Data (7:30 AM)

Start Time	Landwehr Southbound						Techny Westbound						Landwehr Northbound						Techny Eastbound						Int. Total
	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	
7:30 AM	5	76	55	0	0	136	15	29	31	0	0	75	50	67	5	0	0	122	18	86	15	0	0	119	452
7:45 AM	5	61	40	0	0	106	29	26	34	0	0	89	53	112	10	0	3	175	24	74	18	0	0	116	486
8:00 AM	5	62	23	0	0	90	21	40	34	0	0	95	46	111	29	0	3	186	15	60	12	0	0	87	458
8:15 AM	4	63	23	0	0	90	11	31	36	0	0	78	45	97	16	0	4	158	16	57	5	0	0	78	404
Total	19	262	141	0	0	422	76	126	135	0	0	337	194	387	60	0	10	641	73	277	50	0	0	400	1800
Approach %	4.5	62.1	33.4	0.0	-	-	22.6	37.4	40.1	0.0	-	-	30.3	60.4	9.4	0.0	-	-	18.3	69.3	12.5	0.0	-	-	-
Total %	1.1	14.6	7.8	0.0	-	23.4	4.2	7.0	7.5	0.0	-	18.7	10.8	21.5	3.3	0.0	-	35.6	4.1	15.4	2.8	0.0	-	22.2	-
PHF	0.950	0.862	0.641	0.000	-	0.776	0.655	0.788	0.938	0.000	-	0.887	0.915	0.864	0.517	0.000	-	0.862	0.760	0.805	0.694	0.000	-	0.840	0.926
Lights	17	252	138	0	-	407	70	122	135	0	-	327	189	377	57	0	-	623	70	268	49	0	-	387	1744
% Lights	89.5	96.2	97.9	-	-	96.4	92.1	96.8	100.0	-	-	97.0	97.4	97.4	95.0	-	-	97.2	95.9	96.8	98.0	-	-	96.8	96.9
Mediums	2	7	3	0	-	12	4	3	0	0	-	7	4	9	3	0	-	16	2	8	1	0	-	11	46
% Mediums	10.5	2.7	2.1	-	-	2.8	5.3	2.4	0.0	-	-	2.1	2.1	2.3	5.0	-	-	2.5	2.7	2.9	2.0	-	-	2.8	2.6
Articulated Trucks	0	3	0	0	-	3	2	1	0	0	-	3	1	1	0	0	-	2	1	1	0	0	-	2	10
% Articulated Trucks	0.0	1.1	0.0	-	-	0.7	2.6	0.8	0.0	-	-	0.9	0.5	0.3	0.0	-	-	0.3	1.4	0.4	0.0	-	-	0.5	0.6
Bicycles on Crosswalk	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.0	-	-	-	-	-	-	-	
Pedestrians	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	10	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-	-	-	-	-	-

Turning Movement Peak Hour Data (5:00 PM)

Start Time	Landwehr Southbound						Techny Westbound						Landwehr Northbound						Techny Eastbound						Int. Total
	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	Right	Thru	Left	U-Turn	Peds	App. Total	
5:00 PM	10	103	24	0	0	137	28	79	40	0	0	147	28	80	11	0	0	119	14	51	3	0	0	68	471
5:15 PM	9	95	26	0	0	130	36	96	48	0	0	180	22	80	13	0	0	115	10	42	8	0	0	60	485
5:30 PM	8	110	29	0	0	147	27	118	38	0	0	183	21	74	17	0	0	112	18	44	6	0	0	68	510
5:45 PM	8	92	24	0	0	124	47	89	54	0	0	190	21	78	15	0	1	114	20	32	5	0	0	57	485
Total	35	400	103	0	0	538	138	382	180	0	0	700	92	312	56	0	1	460	62	169	22	0	0	253	1951
Approach %	6.5	74.3	19.1	0.0	-	-	19.7	54.6	25.7	0.0	-	-	20.0	67.8	12.2	0.0	-	-	24.5	66.8	8.7	0.0	-	-	-
Total %	1.8	20.5	5.3	0.0	-	27.6	7.1	19.6	9.2	0.0	-	35.9	4.7	16.0	2.9	0.0	-	23.6	3.2	8.7	1.1	0.0	-	13.0	-
PHF	0.875	0.909	0.888	0.000	-	0.915	0.734	0.809	0.833	0.000	-	0.921	0.821	0.975	0.824	0.000	-	0.966	0.775	0.828	0.688	0.000	-	0.930	0.956
Lights	35	390	98	0	-	523	136	379	180	0	-	695	91	307	56	0	-	454	62	167	22	0	-	251	1923
% Lights	100.0	97.5	95.1	-	-	97.2	98.6	99.2	100.0	-	-	99.3	98.9	98.4	100.0	-	-	98.7	100.0	98.8	100.0	-	-	99.2	98.6
Mediums	0	5	4	0	-	9	1	3	0	0	-	4	1	5	0	0	-	6	0	2	0	0	-	2	21
% Mediums	0.0	1.3	3.9	-	-	1.7	0.7	0.8	0.0	-	-	0.6	1.1	1.6	0.0	-	-	1.3	0.0	1.2	0.0	-	-	0.8	1.1
Articulated Trucks	0	5	1	0	-	6	1	0	0	0	-	1	0	0	0	0	-	0	0	0	0	-	0	7	
% Articulated Trucks	0.0	1.3	1.0	-	-	1.1	0.7	0.0	0.0	-	-	0.1	0.0	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.4	
Bicycles on Crosswalk	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	0	-	
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.0	-	-	-	-	-	-		
Pedestrians	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	1	-	-	-	-	0	-	
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-	-	-	-	-	

Gewalt Hamilton Associates Inc.
625 Forest Edge Drive

Vernon Hills, Illinois, United States 60061
(847) 478-9700 poster@gha-engineers.com

Count Name: Landwehr Rd @ North school
drive
Site Code:
Start Date: 11/19/2019
Page No: 1

Turning Movement Data

Start Time	Landwehr Southbound					North School Dr Westbound					Landwehr Northbound					Int. Total
	Thru	Left	U-Turn	Peds	App. Total	Right	Left	U-Turn	Peds	App. Total	Right	Thru	U-Turn	Peds	App. Total	
7:00 AM	82	0	0	0	82	0	0	0	0	0	0	78	0	0	78	160
7:15 AM	116	0	0	0	116	1	0	0	0	1	0	113	0	0	113	230
7:30 AM	123	0	0	0	123	0	1	0	0	1	0	126	0	0	126	250
7:45 AM	118	0	0	0	118	8	2	0	0	10	0	159	0	0	159	287
Hourly Total	439	0	0	0	439	9	3	0	0	12	0	476	0	0	476	927
8:00 AM	113	0	0	0	113	14	20	0	0	34	0	176	0	0	176	323
8:15 AM	110	0	0	0	110	17	28	0	3	45	0	135	0	0	135	290
8:30 AM	112	0	0	0	112	4	5	0	0	9	0	114	0	0	114	235
8:45 AM	106	0	0	0	106	1	3	0	2	4	0	154	0	0	154	264
Hourly Total	441	0	0	0	441	36	56	0	5	92	0	579	0	0	579	1112
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2:30 PM	96	0	0	0	96	0	0	0	0	0	0	68	0	0	68	164
2:45 PM	72	0	0	0	72	0	0	0	0	0	0	111	1	0	112	184
Hourly Total	168	0	0	0	168	0	0	0	0	0	0	179	1	0	180	348
3:00 PM	106	0	0	0	106	0	1	0	0	1	0	89	0	0	89	196
3:15 PM	118	0	0	0	118	0	1	0	1	1	0	84	0	0	84	203
3:30 PM	147	0	0	0	147	19	29	0	2	48	0	120	0	0	120	315
3:45 PM	151	0	0	0	151	1	2	0	0	3	0	107	0	0	107	261
Hourly Total	522	0	0	0	522	20	33	0	3	53	0	400	0	0	400	975
4:00 PM	174	0	0	0	174	1	0	0	1	1	0	99	0	0	99	274
4:15 PM	188	0	0	0	188	1	0	0	0	1	0	117	0	0	117	306
4:30 PM	141	0	0	0	141	4	1	0	0	5	0	119	0	0	119	265
4:45 PM	183	0	0	0	183	7	9	0	0	16	0	84	0	0	84	283
Hourly Total	686	0	0	0	686	13	10	0	1	23	0	419	0	0	419	1128
5:00 PM	154	0	0	0	154	3	2	0	0	5	0	112	0	0	112	271
5:15 PM	152	0	0	0	152	1	0	0	0	1	0	117	0	0	117	270
5:30 PM	166	0	0	0	166	0	0	0	0	0	0	111	0	0	111	277
5:45 PM	169	0	0	0	169	2	1	0	0	3	0	112	0	0	112	284
Hourly Total	641	0	0	0	641	6	3	0	0	9	0	452	0	0	452	1102
Grand Total	2897	0	0	0	2897	84	105	0	9	189	0	2505	1	0	2506	5592
Approach %	100.0	0.0	0.0	-	-	44.4	55.6	0.0	-	-	0.0	100.0	0.0	-	-	-
Total %	51.8	0.0	0.0	-	51.8	1.5	1.9	0.0	-	3.4	0.0	44.8	0.0	-	44.8	-
Lights	2825	0	0	-	2825	80	105	0	-	185	0	2436	1	-	2437	5447
% Lights	97.5	-	-	-	97.5	95.2	100.0	-	-	97.9	-	97.2	100.0	-	97.2	97.4
Mediums	57	0	0	-	57	4	0	0	-	4	0	60	0	-	60	121
% Mediums	2.0	-	-	-	2.0	4.8	0.0	-	-	2.1	-	2.4	0.0	-	2.4	2.2
Articulated Trucks	15	0	0	-	15	0	0	0	-	0	0	9	0	-	9	24
% Articulated Trucks	0.5	-	-	-	0.5	0.0	0.0	-	-	0.0	-	0.4	0.0	-	0.4	0.4
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	-	0	-	-	-	0	-	-

% Bicycles on Crosswalk	-	-	-	-	-	0.0	-	-	-	-	-
Pedestrians	-	-	-	0	-	9	-	-	-	0	-
% Pedestrians	-	-	-	-	-	100.0	-	-	-	-	-

Gewalt Hamilton Associates Inc.
625 Forest Edge Drive

Vernon Hills, Illinois, United States 60061
(847) 478-9700 poster@gha-engineers.com

Count Name: Landwehr Rd @ North school
drive
Site Code:
Start Date: 11/19/2019
Page No: 4

Turning Movement Peak Hour Data (7:30 AM)

Start Time	Landwehr Southbound					North School Dr Westbound					Landwehr Northbound					
	Thru	Left	U-Turn	Peds	App. Total	Right	Left	U-Turn	Peds	App. Total	Right	Thru	U-Turn	Peds	App. Total	Int. Total
7:30 AM	123	0	0	0	123	0	1	0	0	1	0	126	0	0	126	250
7:45 AM	118	0	0	0	118	8	2	0	0	10	0	159	0	0	159	287
8:00 AM	113	0	0	0	113	14	20	0	0	34	0	176	0	0	176	323
8:15 AM	110	0	0	0	110	17	28	0	3	45	0	135	0	0	135	290
Total	464	0	0	0	464	39	51	0	3	90	0	596	0	0	596	1150
Approach %	100.0	0.0	0.0	-	-	43.3	56.7	0.0	-	-	0.0	100.0	0.0	-	-	-
Total %	40.3	0.0	0.0	-	40.3	3.4	4.4	0.0	-	7.8	0.0	51.8	0.0	-	51.8	-
PHF	0.943	0.000	0.000	-	0.943	0.574	0.455	0.000	-	0.500	0.000	0.847	0.000	-	0.847	0.890
Lights	453	0	0	-	453	39	51	0	-	90	0	577	0	-	577	1120
% Lights	97.6	-	-	-	97.6	100.0	100.0	-	-	100.0	-	96.8	-	-	96.8	97.4
Mediums	8	0	0	-	8	0	0	0	-	0	0	17	0	-	17	25
% Mediums	1.7	-	-	-	1.7	0.0	0.0	-	-	0.0	-	2.9	-	-	2.9	2.2
Articulated Trucks	3	0	0	-	3	0	0	0	-	0	0	2	0	-	2	5
% Articulated Trucks	0.6	-	-	-	0.6	0.0	0.0	-	-	0.0	-	0.3	-	-	0.3	0.4
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	0	-	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	0.0	-	-	-	-	-	-	-
Pedestrians	-	-	-	0	-	-	-	-	3	-	-	-	0	-	-	-
% Pedestrians	-	-	-	-	-	-	-	-	100.0	-	-	-	-	-	-	-

Gewalt Hamilton Associates Inc.
625 Forest Edge Drive

Vernon Hills, Illinois, United States 60061
(847) 478-9700 poster@gha-engineers.com

Count Name: Landwehr Rd @ North school
drive
Site Code:
Start Date: 11/19/2019
Page No: 6

Turning Movement Peak Hour Data (3:30 PM)

Start Time	Landwehr Southbound					North School Dr Westbound					Landwehr Northbound					
	Thru	Left	U-Turn	Peds	App. Total	Right	Left	U-Turn	Peds	App. Total	Right	Thru	U-Turn	Peds	App. Total	Int. Total
3:30 PM	147	0	0	0	147	19	29	0	2	48	0	120	0	0	120	315
3:45 PM	151	0	0	0	151	1	2	0	0	3	0	107	0	0	107	261
4:00 PM	174	0	0	0	174	1	0	0	1	1	0	99	0	0	99	274
4:15 PM	188	0	0	0	188	1	0	0	0	1	0	117	0	0	117	306
Total	660	0	0	0	660	22	31	0	3	53	0	443	0	0	443	1156
Approach %	100.0	0.0	0.0	-	-	41.5	58.5	0.0	-	-	0.0	100.0	0.0	-	-	-
Total %	57.1	0.0	0.0	-	57.1	1.9	2.7	0.0	-	4.6	0.0	38.3	0.0	-	38.3	-
PHF	0.878	0.000	0.000	-	0.878	0.289	0.267	0.000	-	0.276	0.000	0.923	0.000	-	0.923	0.917
Lights	639	0	0	-	639	21	31	0	-	52	0	431	0	-	431	1122
% Lights	96.8	-	-	-	96.8	95.5	100.0	-	-	98.1	-	97.3	-	-	97.3	97.1
Mediums	17	0	0	-	17	1	0	0	-	1	0	11	0	-	11	29
% Mediums	2.6	-	-	-	2.6	4.5	0.0	-	-	1.9	-	2.5	-	-	2.5	2.5
Articulated Trucks	4	0	0	-	4	0	0	0	-	0	0	1	0	-	1	5
% Articulated Trucks	0.6	-	-	-	0.6	0.0	0.0	-	-	0.0	-	0.2	-	-	0.2	0.4
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	0	-	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	0.0	-	-	-	-	-	-	-
Pedestrians	-	-	-	0	-	-	-	-	3	-	-	-	0	-	-	-
% Pedestrians	-	-	-	-	-	-	-	-	100.0	-	-	-	-	-	-	-

Turning Movement Data

Start Time	Landwehr Southbound					South School Dr Westbound					Landwehr Northbound					Int. Total
	Thru	Left	U-Turn	Peds	App. Total	Right	Left	U-Turn	Peds	App. Total	Right	Thru	U-Turn	Peds	App. Total	
7:00 AM	88	0	0	0	88	0	0	0	0	0	0	79	0	0	79	167
7:15 AM	117	0	0	0	117	0	0	0	0	0	1	115	0	0	116	233
7:30 AM	122	1	0	0	123	0	0	0	0	0	2	133	0	0	135	258
7:45 AM	119	1	0	0	120	0	0	0	0	0	10	154	0	0	164	284
Hourly Total	446	2	0	0	448	0	0	0	0	0	13	481	0	0	494	942
8:00 AM	125	5	0	0	130	0	0	0	2	0	34	174	0	0	208	338
8:15 AM	132	9	0	0	141	0	0	0	1	0	32	140	0	0	172	313
8:30 AM	120	3	0	0	123	0	0	0	2	0	6	119	0	0	125	248
8:45 AM	108	1	0	0	109	0	0	0	0	0	2	144	0	0	146	255
Hourly Total	485	18	0	0	503	0	0	0	5	0	74	577	0	0	651	1154
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2:30 PM	98	0	0	0	98	0	0	0	0	0	0	68	0	0	68	166
2:45 PM	69	3	0	0	72	0	0	0	0	0	7	114	0	0	121	193
Hourly Total	167	3	0	0	170	0	0	0	0	0	7	182	0	0	189	359
3:00 PM	102	6	0	0	108	0	0	0	0	0	8	89	0	0	97	205
3:15 PM	115	4	0	0	119	0	0	0	0	0	4	83	0	0	87	206
3:30 PM	172	2	0	0	174	0	0	0	2	0	16	120	0	0	136	310
3:45 PM	155	0	0	0	155	0	0	0	1	0	2	108	0	0	110	265
Hourly Total	544	12	0	0	556	0	0	0	3	0	30	400	0	0	430	986
4:00 PM	174	1	0	0	175	0	0	0	0	0	3	100	0	0	103	278
4:15 PM	184	2	0	0	186	0	0	0	0	0	1	117	0	0	118	304
4:30 PM	140	4	0	0	144	0	0	0	0	0	9	121	0	0	130	274
4:45 PM	198	1	0	0	199	0	0	0	0	0	4	84	0	0	88	287
Hourly Total	696	8	0	0	704	0	0	0	0	0	17	422	0	0	439	1143
5:00 PM	160	1	0	0	161	0	0	0	0	0	1	111	0	0	112	273
5:15 PM	157	1	0	0	158	0	0	0	0	0	1	117	0	0	118	276
5:30 PM	164	0	0	0	164	0	0	0	0	0	0	109	0	0	109	273
5:45 PM	162	1	0	0	163	0	0	0	0	0	0	113	0	0	113	276
Hourly Total	643	3	0	0	646	0	0	0	0	0	2	450	0	0	452	1098
Grand Total	2981	46	0	0	3027	0	0	0	8	0	143	2512	0	0	2655	5682
Approach %	98.5	1.5	0.0	-	-	0.0	0.0	0.0	-	-	5.4	94.6	0.0	-	-	-
Total %	52.5	0.8	0.0	-	53.3	0.0	0.0	0.0	-	0.0	2.5	44.2	0.0	-	46.7	-
Lights	2912	45	0	-	2957	0	0	0	-	0	140	2443	0	-	2583	5540
% Lights	97.7	97.8	-	-	97.7	-	-	-	-	-	97.9	97.3	-	-	97.3	97.5
Mediums	53	1	0	-	54	0	0	0	-	0	3	59	0	-	62	116
% Mediums	1.8	2.2	-	-	1.8	-	-	-	-	-	2.1	2.3	-	-	2.3	2.0
Articulated Trucks	16	0	0	-	16	0	0	0	-	0	0	10	0	-	10	26
% Articulated Trucks	0.5	0.0	-	-	0.5	-	-	-	-	-	0.0	0.4	-	-	0.4	0.5
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	-	-	-	-	0	-	-	-

% Bicycles on Crosswalk	-	-	-	-	-	0.0	-	-	-	-	-	-
Pedestrians	-	-	-	0	-	8	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	100.0	-	-	-	-	-	-

Gewalt Hamilton Associates Inc.
625 Forest Edge Drive

Vernon Hills, Illinois, United States 60061
(847) 478-9700 poster@gha-engineers.com

Count Name: Landwehr Rd @ South school
drive
Site Code:
Start Date: 11/19/2019
Page No: 4

Turning Movement Peak Hour Data (7:30 AM)

Start Time	Landwehr Southbound					South School Dr Westbound					Landwehr Northbound					Int. Total
	Thru	Left	U-Turn	Peds	App. Total	Right	Left	U-Turn	Peds	App. Total	Right	Thru	U-Turn	Peds	App. Total	
7:30 AM	122	1	0	0	123	0	0	0	0	0	2	133	0	0	135	258
7:45 AM	119	1	0	0	120	0	0	0	0	0	10	154	0	0	164	284
8:00 AM	125	5	0	0	130	0	0	0	2	0	34	174	0	0	208	338
8:15 AM	132	9	0	0	141	0	0	0	1	0	32	140	0	0	172	313
Total	498	16	0	0	514	0	0	0	3	0	78	601	0	0	679	1193
Approach %	96.9	3.1	0.0	-	-	0.0	0.0	0.0	-	-	11.5	88.5	0.0	-	-	-
Total %	41.7	1.3	0.0	-	43.1	0.0	0.0	0.0	-	0.0	6.5	50.4	0.0	-	56.9	-
PHF	0.943	0.444	0.000	-	0.911	0.000	0.000	0.000	-	0.000	0.574	0.864	0.000	-	0.816	0.882
Lights	486	16	0	-	502	0	0	0	-	0	78	582	0	-	660	1162
% Lights	97.6	100.0	-	-	97.7	-	-	-	-	-	100.0	96.8	-	-	97.2	97.4
Mediums	9	0	0	-	9	0	0	0	-	0	0	17	0	-	17	26
% Mediums	1.8	0.0	-	-	1.8	-	-	-	-	-	0.0	2.8	-	-	2.5	2.2
Articulated Trucks	3	0	0	-	3	0	0	0	-	0	0	2	0	-	2	5
% Articulated Trucks	0.6	0.0	-	-	0.6	-	-	-	-	-	0.0	0.3	-	-	0.3	0.4
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	0	-	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	0.0	-	-	-	-	-	-	-
Pedestrians	-	-	-	0	-	-	-	-	3	-	-	-	0	-	-	-
% Pedestrians	-	-	-	-	-	-	-	-	100.0	-	-	-	-	-	-	-

Gewalt Hamilton Associates Inc.
625 Forest Edge Drive

Vernon Hills, Illinois, United States 60061
(847) 478-9700 poster@gha-engineers.com

Count Name: Landwehr Rd @ South school
drive
Site Code:
Start Date: 11/19/2019
Page No: 6

Turning Movement Peak Hour Data (3:30 PM)

Start Time	Landwehr Southbound					South School Dr Westbound					Landwehr Northbound					Int. Total
	Thru	Left	U-Turn	Peds	App. Total	Right	Left	U-Turn	Peds	App. Total	Right	Thru	U-Turn	Peds	App. Total	
3:30 PM	172	2	0	0	174	0	0	0	2	0	16	120	0	0	136	310
3:45 PM	155	0	0	0	155	0	0	0	1	0	2	108	0	0	110	265
4:00 PM	174	1	0	0	175	0	0	0	0	0	3	100	0	0	103	278
4:15 PM	184	2	0	0	186	0	0	0	0	0	1	117	0	0	118	304
Total	685	5	0	0	690	0	0	0	3	0	22	445	0	0	467	1157
Approach %	99.3	0.7	0.0	-	-	0.0	0.0	0.0	-	-	4.7	95.3	0.0	-	-	-
Total %	59.2	0.4	0.0	-	59.6	0.0	0.0	0.0	-	0.0	1.9	38.5	0.0	-	40.4	-
PHF	0.931	0.625	0.000	-	0.927	0.000	0.000	0.000	-	0.000	0.344	0.927	0.000	-	0.858	0.933
Lights	663	5	0	-	668	0	0	0	-	0	21	432	0	-	453	1121
% Lights	96.8	100.0	-	-	96.8	-	-	-	-	-	95.5	97.1	-	-	97.0	96.9
Mediums	17	0	0	-	17	0	0	0	-	0	1	11	0	-	12	29
% Mediums	2.5	0.0	-	-	2.5	-	-	-	-	-	4.5	2.5	-	-	2.6	2.5
Articulated Trucks	5	0	0	-	5	0	0	0	-	0	0	2	0	-	2	7
% Articulated Trucks	0.7	0.0	-	-	0.7	-	-	-	-	-	0.0	0.4	-	-	0.4	0.6
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	0.0	-	-	-	-	-	-	-
Pedestrians	-	-	-	0	-	-	-	-	3	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	100.0	-	-	-	-	-	-	-

Turning Movement Data

Start Time	Techny Westbound					West School Dr Northbound					Techny Eastbound					Int. Total
	Thru	Left	U-Turn	Peds	App. Total	Right	Left	U-Turn	Peds	App. Total	Right	Thru	U-Turn	Peds	App. Total	
7:00 AM	57	1	0	0	58	0	0	0	0	0	2	87	0	0	89	147
7:15 AM	77	5	0	0	82	1	1	0	0	2	8	142	0	0	150	234
7:30 AM	71	3	0	0	74	1	0	0	0	1	22	173	0	0	195	270
7:45 AM	93	4	0	0	97	1	1	0	0	2	21	148	0	0	169	268
Hourly Total	298	13	0	0	311	3	2	0	0	5	53	550	0	0	603	919
8:00 AM	90	4	0	0	94	0	1	0	0	1	6	122	0	0	128	223
8:15 AM	72	4	0	0	76	3	1	0	0	4	14	111	0	0	125	205
8:30 AM	100	0	0	0	100	0	0	0	0	0	3	103	0	0	106	206
8:45 AM	74	1	0	0	75	0	0	0	0	0	3	92	0	0	95	170
Hourly Total	336	9	0	0	345	3	2	0	0	5	26	428	0	0	454	804
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2:30 PM	93	0	0	0	93	1	1	0	0	2	1	58	0	0	59	154
2:45 PM	81	0	0	0	81	0	0	0	0	0	3	72	0	0	75	156
Hourly Total	174	0	0	0	174	1	1	0	0	2	4	130	0	0	134	310
3:00 PM	102	3	0	0	105	0	0	0	0	0	1	60	0	0	61	166
3:15 PM	137	3	0	0	140	1	0	0	0	1	11	70	0	0	81	222
3:30 PM	156	2	0	0	158	0	5	0	0	5	10	75	0	0	85	248
3:45 PM	143	3	0	0	146	1	1	0	0	2	7	72	0	0	79	227
Hourly Total	538	11	0	0	549	2	6	0	0	8	29	277	0	0	306	863
4:00 PM	144	2	0	0	146	2	0	0	0	2	4	75	0	0	79	227
4:15 PM	153	1	0	0	154	1	0	0	0	1	3	76	0	0	79	234
4:30 PM	141	1	0	0	142	0	2	0	0	2	7	76	0	0	83	227
4:45 PM	159	3	0	0	162	5	9	0	0	14	4	86	0	0	90	266
Hourly Total	597	7	0	0	604	8	11	0	0	19	18	313	0	0	331	954
5:00 PM	135	1	0	0	136	0	0	0	0	0	6	99	0	0	105	241
5:15 PM	177	2	0	0	179	2	3	0	0	5	3	89	0	0	92	276
5:30 PM	180	1	0	0	181	0	1	0	0	1	0	94	0	0	94	276
5:45 PM	179	2	0	0	181	3	12	0	0	15	2	74	0	0	76	272
Hourly Total	671	6	0	0	677	5	16	0	0	21	11	356	0	0	367	1065
Grand Total	2614	46	0	0	2660	22	38	0	0	60	141	2054	0	0	2195	4915
Approach %	98.3	1.7	0.0	-	-	36.7	63.3	0.0	-	-	6.4	93.6	0.0	-	-	-
Total %	53.2	0.9	0.0	-	54.1	0.4	0.8	0.0	-	1.2	2.9	41.8	0.0	-	44.7	-
Lights	2556	43	0	-	2599	22	38	0	-	60	125	2003	0	-	2128	4787
% Lights	97.8	93.5	-	-	97.7	100.0	100.0	-	-	100.0	88.7	97.5	-	-	96.9	97.4
Mediums	45	3	0	-	48	0	0	0	-	0	16	44	0	-	60	108
% Mediums	1.7	6.5	-	-	1.8	0.0	0.0	-	-	0.0	11.3	2.1	-	-	2.7	2.2
Articulated Trucks	13	0	0	-	13	0	0	0	-	0	0	7	0	-	7	20
% Articulated Trucks	0.5	0.0	-	-	0.5	0.0	0.0	-	-	0.0	0.0	0.3	-	-	0.3	0.4
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-

Gewalt Hamilton Associates Inc.
625 Forest Edge Drive

Vernon Hills, Illinois, United States 60061
(847) 478-9700 poster@gha-engineers.com

Count Name: Techny Rd @ West school drive
Site Code:
Start Date: 11/19/2019
Page No: 4

Turning Movement Peak Hour Data (7:15 AM)

Gewalt Hamilton Associates Inc.
625 Forest Edge Drive

Vernon Hills, Illinois, United States 60061
(847) 478-9700 poster@gha-engineers.com

Count Name: Techny Rd @ West school drive
Site Code:
Start Date: 11/19/2019
Page No: 6

Turning Movement Peak Hour Data (5:00 PM)

Turning Movement Data

Start Time	Techny Westbound					East School Dr Northbound					Techny Eastbound					Int. Total
	Thru	Left	U-Turn	Peds	App. Total	Right	Left	U-Turn	Peds	App. Total	Right	Thru	U-Turn	Peds	App. Total	
7:00 AM	55	1	0	0	56	0	0	0	0	0	0	87	0	0	87	143
7:15 AM	76	0	0	0	76	3	3	0	0	6	0	144	0	0	144	226
7:30 AM	62	0	0	0	62	3	12	0	0	15	0	176	0	0	176	253
7:45 AM	91	0	0	0	91	2	7	0	0	9	0	148	0	0	148	248
Hourly Total	284	1	0	0	285	8	22	0	0	30	0	555	0	0	555	870
8:00 AM	94	0	0	0	94	2	0	0	0	2	0	122	0	0	122	218
8:15 AM	76	1	0	0	77	2	2	0	0	4	1	114	0	0	115	196
8:30 AM	98	0	0	0	98	2	3	0	0	5	0	104	0	0	104	207
8:45 AM	74	0	0	0	74	1	1	0	0	2	0	92	0	0	92	168
Hourly Total	342	1	0	0	343	7	6	0	0	13	1	432	0	0	433	789
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
2:30 PM	96	0	0	0	96	0	0	0	0	0	0	58	0	0	58	154
2:45 PM	77	0	0	0	77	1	2	0	0	3	1	69	0	0	70	150
Hourly Total	173	0	0	0	173	1	2	0	0	3	1	127	0	0	128	304
3:00 PM	103	0	0	0	103	1	1	0	0	2	0	58	0	0	58	163
3:15 PM	138	0	0	0	138	0	4	0	0	4	1	69	0	0	70	212
3:30 PM	146	0	0	0	146	7	14	0	0	21	0	75	0	0	75	242
3:45 PM	132	0	0	0	132	5	12	0	2	17	0	73	0	0	73	222
Hourly Total	519	0	0	0	519	13	31	0	2	44	1	275	0	0	276	839
4:00 PM	147	0	0	0	147	0	3	0	3	3	0	75	0	0	75	225
4:15 PM	153	0	0	0	153	0	1	0	0	1	0	79	0	0	79	233
4:30 PM	143	0	0	0	143	1	3	0	0	4	1	75	0	0	76	223
4:45 PM	155	0	0	0	155	2	5	0	0	7	1	87	0	0	88	250
Hourly Total	598	0	0	0	598	3	12	0	3	15	2	316	0	0	318	931
5:00 PM	141	0	0	0	141	1	2	0	0	3	1	96	0	0	97	241
5:15 PM	183	0	0	0	183	1	2	0	0	3	0	90	0	0	90	276
5:30 PM	181	0	0	0	181	2	1	0	0	3	0	94	0	0	94	278
5:45 PM	171	0	0	0	171	6	5	0	0	11	0	79	0	0	79	261
Hourly Total	676	0	0	0	676	10	10	0	0	20	1	359	0	0	360	1056
Grand Total	2592	2	0	0	2594	42	83	0	5	125	6	2064	0	0	2070	4789
Approach %	99.9	0.1	0.0	-	-	33.6	66.4	0.0	-	-	0.3	99.7	0.0	-	-	-
Total %	54.1	0.0	0.0	-	54.2	0.9	1.7	0.0	-	2.6	0.1	43.1	0.0	-	43.2	-
Lights	2543	2	0	-	2545	30	77	0	-	107	6	2011	0	-	2017	4669
% Lights	98.1	100.0	-	-	98.1	71.4	92.8	-	-	85.6	100.0	97.4	-	-	97.4	97.5
Mediums	43	0	0	-	43	12	6	0	-	18	0	43	0	-	43	104
% Mediums	1.7	0.0	-	-	1.7	28.6	7.2	-	-	14.4	0.0	2.1	-	-	2.1	2.2
Articulated Trucks	6	0	0	-	6	0	0	0	-	0	0	10	0	-	10	16
% Articulated Trucks	0.2	0.0	-	-	0.2	0.0	0.0	-	-	0.0	0.0	0.5	-	-	0.5	0.3
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	-	0	-	-	-	0	-	-

% Bicycles on Crosswalk	-	-	-	-	-	0.0	-	-	-	-	-
Pedestrians	-	-	-	0	-	-	-	-	-	0	-
% Pedestrians	-	-	-	-	-	-	100.0	-	-	-	-

Gewalt Hamilton Associates Inc.
625 Forest Edge Drive

Vernon Hills, Illinois, United States 60061
(847) 478-9700 poster@gha-engineers.com

Count Name: 1) Techny Road @ Techny Rd @
East school drive
Site Code:
Start Date: 11/19/2019
Page No: 4

Turning Movement Peak Hour Data (7:15 AM)

Gewalt Hamilton Associates Inc.
625 Forest Edge Drive

Vernon Hills, Illinois, United States 60061
(847) 478-9700 poster@gha-engineers.com

Count Name: 1) Techny Road @ Techny Rd @
East school drive
Site Code:
Start Date: 11/19/2019
Page No: 6

Turning Movement Peak Hour Data (5:00 PM)

APPENDIX C

Crash Summary

Appendix C
Crash Data Summary
Landwehr Road at Techny Road, Northbrook, IL

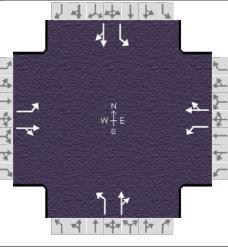
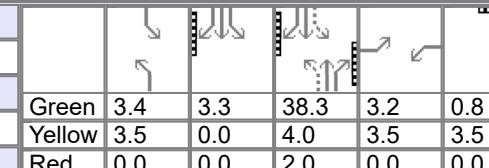
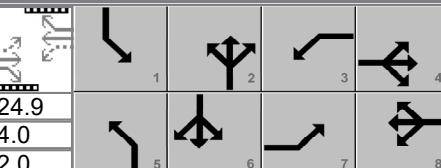
Type of Crash	2014		2015		2016		2017		2018	
	No.	%	No.	%	No.	%	No.	%	No.	%
	-	-	-	-	-	-	-	-	-	-
Angle	1	17%	-	-	-	-	1	20%	1	14%
Turning	1	17%	-	-	-	-	1	20%	-	-
Rear End	4	66%	1	100%	3	100%	2	40%	6	86%
Sideswipe Same Direction	-	-	-	-	-	-	1	20%	-	-
Pavement Conditions										
Snowy or Icy Conditions	-	-	-	-	-	-	-	-	2	29%
Wet Conditions	-	-	-	-	1	33%	-	-	1	14%
Dry Conditions	6	100%	1	100%	2	67%	5	100%	4	57%
Light Conditions										
Dawn or Dusk Conditions	-	-	-	-	-	-	-	-	2	29%
Night Conditions	2	33%	-	-	-	-	-	-	2	29%
Day Conditions	4	67%	1	100%	3	100%	5	100%	3	42%
Crash Severity										
Fatal	-	-	-	-	-	-	-	-	-	-
Injury	-	-	-	-	-	-	-	-	-	-
-A (Incapacitating)	-	-	-	-	-	-	-	-	-	-
-B (Non-incapacitating)	-	-	-	-	-	-	-	-	2	29%
-C (Reported/Not evident)	4	67%	-	-	1	33%	2	40%	1	14%
Property Damage Only	2	33%	1	100%	2	67%	3	60%	4	57%
Total Crashes	6		1		3		5		7	

5-Year Crash Summary	2014-2018	
	No.	%
	-	-
Total Crashes	22	
Type of Crash		
-Angle	3	14%
-Turning	2	9%
-Rear End	16	73%
-Sideswipe Same Direction	1	4%
Pavement Condition		
-Dry	18	82%
-Wet/Snow/Ice	4	18%
Light Condition		
-Daylight	16	73%
-Dark	6	27%
Crash Severity		
-Fatal	0	0%
-Injury	10	45%
-PDO	12	55%

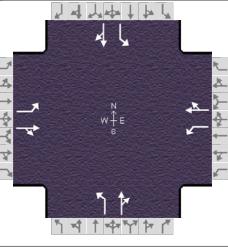
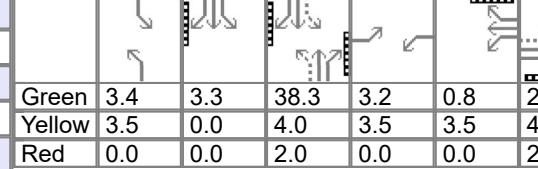
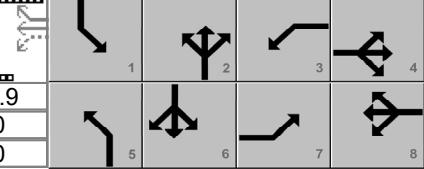
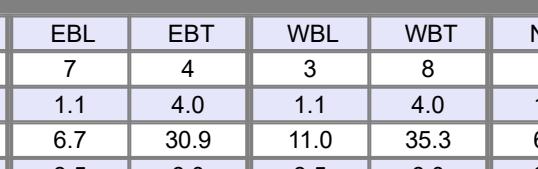
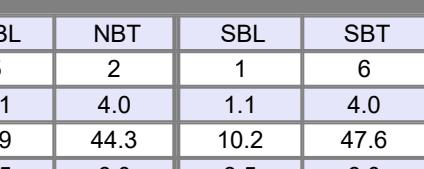
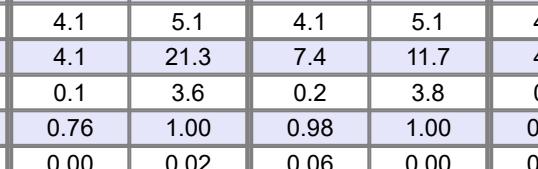
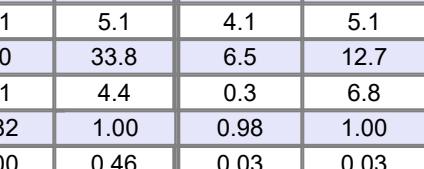
APPENDIX D

Capacity Analysis Worksheets

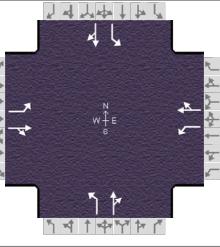
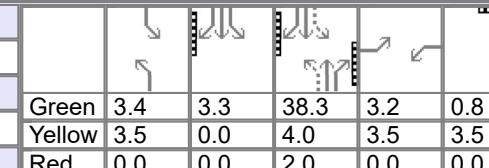
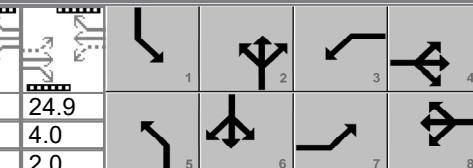
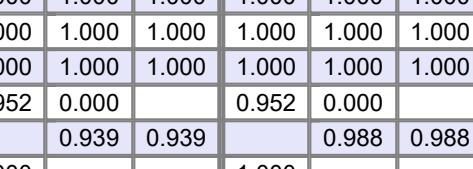
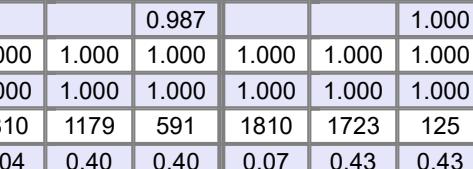
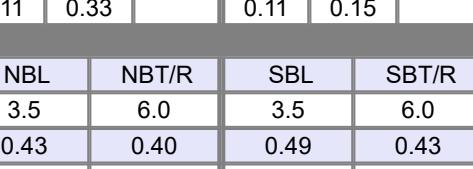
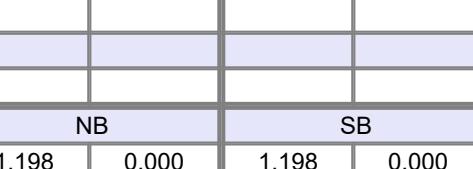
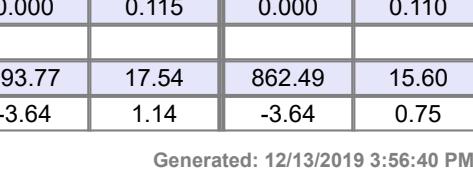
HCS7 Signalized Intersection Input Data

General Information						Intersection Information												
Agency	GHA					Duration, h	0.250											
Analyst	TM	Analysis Date	12/12/2019		Area Type	Other												
Jurisdiction	CDDOT	Time Period	7:30-8:30 AM		PHF	0.93												
Urban Street	Landwehr Road	Analysis Year	2019		Analysis Period	1 > 7:30												
Intersection	Landwehr at Techny		File Name	AM Landwehr at Techny.xus														
Project Description	AM Peak																	
Demand Information				EB		WB		NB		SB								
Approach Movement				L	T	R	L	T	R	L	T	R						
Demand (v), veh/h				50	283	73	135	126	76	60	387	194						
Signal Information																		
Cycle, s	96.4	Reference Phase	2	Green	3.4	3.3	38.3	3.2	0.8	24.9								
Offset, s	0	Reference Point	End	Yellow	3.5	0.0	4.0	3.5	3.5	4.0								
Uncoordinated	Yes	Simult. Gap E/W	On	Red	0.0	0.0	2.0	0.0	0.0	2.0								
Force Mode	Fixed	Simult. Gap N/S	On															
Traffic Information				EB		WB		NB		SB								
Approach Movement				L	T	R	L	T	R	L	T	R						
Demand (v), veh/h				50	283	73	135	126	76	60	387	194						
Initial Queue (Q _b), veh/h				0	0	0	0	0	0	0	0	0						
Base Saturation Flow Rate (s ₀), veh/h				1900	1900	1900	1900	1900	1900	1900	1900	1900						
Parking (N _m), man/h					None			None			None							
Heavy Vehicles (P _{HV}), %				0	2		0	4		0	1							
Ped / Bike / RTOR, /h				0	0	0	0	0	0	10	0	0						
Buses (N _b), buses/h				0	0	0	0	0	0	0	0	0						
Arrival Type (AT)				3	3	3	3	3	3	3	3	3						
Upstream Filtering (I)				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00						
Lane Width (W), ft				12.0	12.0		12.0	12.0		12.0	12.0							
Turn Bay Length, ft				160	0		190	0		130	0							
Grade (Pg), %					0			0			0							
Speed Limit, mi/h				30	30	30	30	30	30	30	30	30						
Phase Information				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT							
Maximum Green (G _{max}) or Phase Split, s				15.0	45.0	15.0	45.0	15.0	45.0	15.0	45.0							
Yellow Change Interval (Y), s				3.5	4.0	3.5	4.0	3.5	4.0	3.5	4.0							
Red Clearance Interval (R _c), s				0.0	2.0	0.0	2.0	0.0	2.0	0.0	2.0							
Minimum Green (G _{min}), s				3	8	3	8	3	8	3	8							
Start-Up Lost Time (It), s				2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0							
Extension of Effective Green (e), s				2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0							
Passage (PT), s				3.0	4.0	3.0	4.0	3.0	4.0	3.0	4.0							
Recall Mode				Off	Min	Off	Min	Off	Min	Off	Min							
Dual Entry				No	Yes	No	Yes	No	Yes	No	Yes							
Walk (Walk), s					7.0		7.0		7.0		7.0							
Pedestrian Clearance Time (PC), s					14.0		14.0		14.0		14.0							
Multimodal Information				EB		WB		NB		SB								
85th % Speed / Rest in Walk / Corner Radius				0	No	25	0	No	25	0	No	25						
Walkway / Crosswalk Width / Length, ft				9.0	12	0	9.0	12	0	9.0	12	0						
Street Width / Island / Curb				0	0	No	0	0	No	0	0	No						
Width Outside / Bike Lane / Shoulder, ft				12	5.0	2.0	12	5.0	2.0	12	5.0	2.0						
Pedestrian Signal / Occupied Parking				No	0.50		No	0.50		No	0.50							

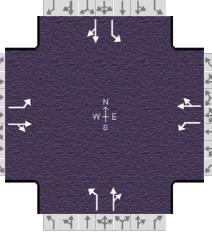
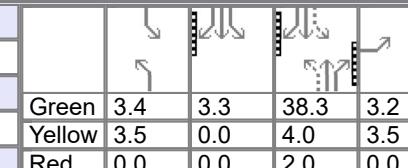
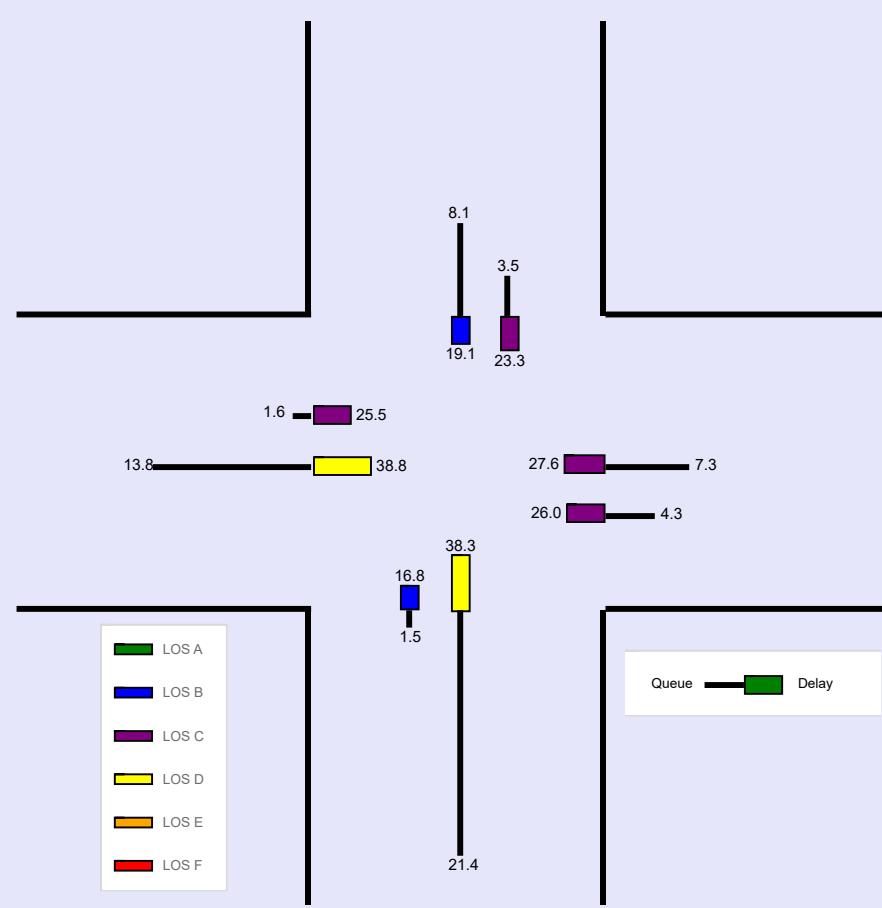
HCS7 Signalized Intersection Results Summary

General Information						Intersection Information						
Agency	GHA			Duration, h			0.250					
Analyst	TM		Analysis Date	12/12/2019		Area Type		Other				
Jurisdiction	CCDOHT		Time Period	7:30-8:30 AM		PHF		0.93				
Urban Street	Landwehr Road		Analysis Year	2019		Analysis Period		1 > 7:30				
Intersection	Landwehr at Techny		File Name	AM Landwehr at Techny.xus								
Project Description	AM Peak											
Demand Information			EB		WB		NB		SB			
Approach Movement			L	T	R	L	T	R	L			
Demand (v), veh/h			50	283	73	135	126	76	60			
									262			
									19			
Signal Information												
Cycle, s	96.4	Reference Phase	2									
Offset, s	0	Reference Point	End		Green	3.4	3.3	38.3	3.2			
Uncoordinated	Yes	Simult. Gap E/W	On		Yellow	3.5	0.0	4.0	3.5			
Force Mode	Fixed	Simult. Gap N/S	On		Red	0.0	0.0	2.0	0.0			
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	
Assigned Phase				7	4	3	8	5	2	1	6	
Case Number				1.1	4.0	1.1	4.0	1.1	4.0	1.1	4.0	
Phase Duration, s				6.7	30.9	11.0	35.3	6.9	44.3	10.2	47.6	
Change Period, (Y+R _c), s				3.5	6.0	3.5	6.0	3.5	6.0	3.5	6.0	
Max Allow Headway (MAH), s				4.1	5.1	4.1	5.1	4.1	5.1	4.1	5.1	
Queue Clearance Time (g _s), s				4.1	21.3	7.4	11.7	4.0	33.8	6.5	12.7	
Green Extension Time (g _e), s				0.1	3.6	0.2	3.8	0.1	4.4	0.3	6.8	
Phase Call Probability				0.76	1.00	0.98	1.00	0.82	1.00	0.98	1.00	
Max Out Probability				0.00	0.02	0.06	0.00	0.00	0.46	0.03	0.03	
Movement Group Results				EB		WB		NB		SB		
Approach Movement				L	T	R	L	T	R	L	T	R
Assigned Movement				7	4	14	3	8	18	5	2	12
Adjusted Flow Rate (v), veh/h				54	383		145	217		65	625	
Adjusted Saturation Flow Rate (s), veh/h/ln				1810	1804		1810	1724		1810	1770	
Queue Service Time (g _s), s				2.1	19.3		5.4	9.7		2.0	31.8	
Cycle Queue Clearance Time (g _c), s				2.1	19.3		5.4	9.7		2.0	31.8	
Green Ratio (g/C)				0.29	0.26		0.36	0.30		0.43	0.40	
Capacity (c), veh/h				351	467		276	523		467	703	
Volume-to-Capacity Ratio (X)				0.153	0.820		0.527	0.415		0.138	0.889	
Back of Queue (Q), ft/ln (95 th percentile)				40.9	351.4		107.2	187.5		37.8	538.6	
Back of Queue (Q), veh/ln (95 th percentile)				1.6	13.8		4.3	7.3		1.5	21.4	
Queue Storage Ratio (RQ) (95 th percentile)				0.26	0.00		0.56	0.00		0.29	0.00	
Uniform Delay (d ₁), s/veh				25.3	33.7		24.4	26.8		16.6	27.1	
Incremental Delay (d ₂), s/veh				0.2	5.1		1.6	0.7		0.1	11.2	
Initial Queue Delay (d ₃), s/veh				0.0	0.0		0.0	0.0		0.0	0.0	
Control Delay (d), s/veh				25.5	38.8		26.0	27.6		16.8	38.3	
Level of Service (LOS)				C	D		C	C		B	D	
Approach Delay, s/veh / LOS				37.1	D		26.9	C		36.3	D	
Intersection Delay, s/veh / LOS				31.0				C				
Multimodal Results				EB		WB		NB		SB		
Pedestrian LOS Score / LOS				1.93	B		1.92	B		1.91	B	
Bicycle LOS Score / LOS				1.21	A		1.09	A		1.62	B	
										1.24	A	

HCS7 Signalized Intersection Intermediate Values

General Information							Intersection Information														
Agency	GHA			Duration, h	0.250																
Analyst	TM		Analysis Date	12/12/2019		Area Type	Other														
Jurisdiction	CCDOHT		Time Period	7:30-8:30 AM		PHF	0.93														
Urban Street	Landwehr Road		Analysis Year	2019		Analysis Period	1 > 7:30														
Intersection	Landwehr at Techny		File Name	AM Landwehr at Techny.xus																	
Project Description	AM Peak																				
Demand Information				EB		WB		NB		SB											
Approach Movement				L	T	R	L	T	R	L	T	R									
Demand (v), veh/h				50	283	73	135	126	76	60	387	194									
				141	262	19															
Signal Information																					
Cycle, s	96.4	Reference Phase	2																		
Offset, s	0	Reference Point	End		Green	3.4	3.3	38.3	3.2	0.8	24.9										
Uncoordinated	Yes	Simult. Gap E/W	On		Yellow	3.5	0.0	4.0	3.5	3.5	4.0										
Force Mode	Fixed	Simult. Gap N/S	On		Red	0.0	0.0	2.0	0.0	0.0	2.0										
Saturation Flow / Delay				L	T	R	L	T	R	L	T	R									
Lane Width Adjustment Factor (f_w)	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000									
Heavy Vehicles and Grade Factor (f_{Hvg})	1.000	0.984	1.000	1.000	0.969	1.000	1.000	0.992	1.000	1.000	0.984	1.000									
Parking Activity Adjustment Factor (f_p)	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000									
Bus Blockage Adjustment Factor (f_{bb})	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000									
Area Type Adjustment Factor (f_a)	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000									
Lane Utilization Adjustment Factor (f_{LU})	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000									
Left-Turn Adjustment Factor (f_{LT})	0.952	0.000		0.952	0.000		0.952	0.000		0.952	0.000										
Right-Turn Adjustment Factor (f_{RT})		0.964	0.964		0.937	0.937		0.939	0.939		0.988	0.988									
Left-Turn Pedestrian Adjustment Factor (f_{Lpb})	1.000			1.000			1.000			1.000											
Right-Turn Ped-Bike Adjustment Factor (f_{Rpb})			1.000			1.000			0.987			1.000									
Work Zone Adjustment Factor (f_{wz})	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000									
DDI Factor (f_{DDI})	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000									
Movement Saturation Flow Rate (s), veh/h	1810	1434	370	1810	1075	649	1810	1179	591	1810	1723	125									
Proportion of Vehicles Arriving on Green (P)	0.03	0.26	0.26	0.08	0.30	0.30	0.04	0.40	0.40	0.07	0.43	0.43									
Incremental Delay Factor (k)	0.11	0.15		0.11	0.15		0.11	0.33		0.11	0.15										
Signal Timing / Movement Groups				EBL	EBT/R	WBL	WBT/R	NBL	NBT/R	SBL	SBT/R										
Lost Time (t_L)	3.5	6.0		3.5	6.0		3.5	6.0		3.5	6.0										
Green Ratio (g/C)	0.29	0.26		0.36	0.30		0.43	0.40		0.49	0.43										
Permitted Saturation Flow Rate (s_p), veh/h/ln	1182	0		1016	0		1094	0		813	0										
Shared Saturation Flow Rate (s_{sh}), veh/h/ln																					
Permitted Effective Green Time (g_p), s	25.0	0.0		27.0	0.0		38.3	0.0		40.2	0.0										
Permitted Service Time (g_u), s	17.7	0.0		5.7	0.0		29.0	0.0		6.5	0.0										
Permitted Queue Service Time (g_{ps}), s	0.3			3.5			0.6			6.5											
Time to First Blockage (g_f), s	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0										
Queue Service Time Before Blockage (g_{fs}), s																					
Protected Right Saturation Flow (s_R), veh/h/ln																					
Protected Right Effective Green Time (g_R), s																					
Multimodal				EB		WB		NB		SB											
Pedestrian F_w / F_v	1.198	0.000		1.198	0.000		1.198	0.000		1.198	0.000										
Pedestrian F_s / F_{delay}	0.000	0.131		0.000	0.126		0.000	0.115		0.000	0.110										
Pedestrian M_{corner} / M_{cw}																					
Bicycle c_b / d_b	517.09	26.51		606.72	23.40		793.77	17.54		862.49	15.60										
Bicycle F_w / F_v	-3.64	0.72		-3.64	0.60		-3.64	1.14		-3.64	0.75										

HCS7 Signalized Intersection Results Graphical Summary

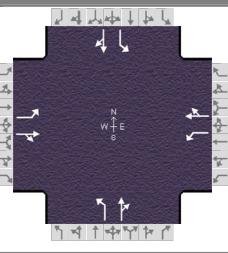
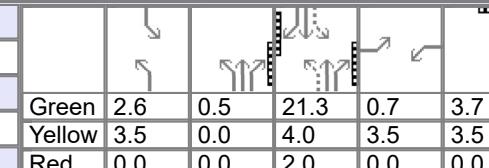
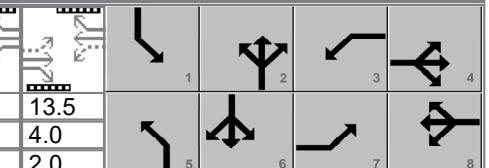
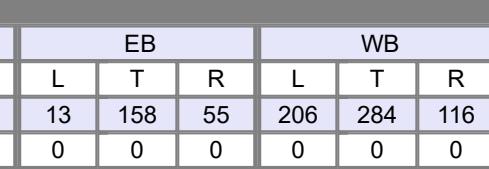
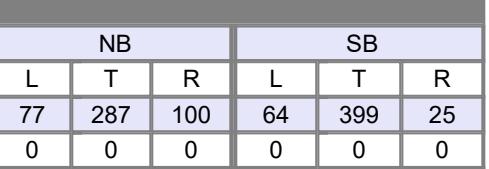
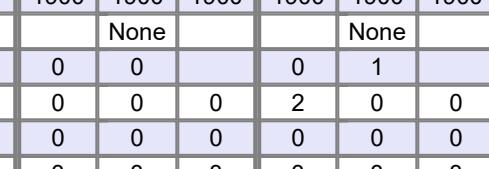
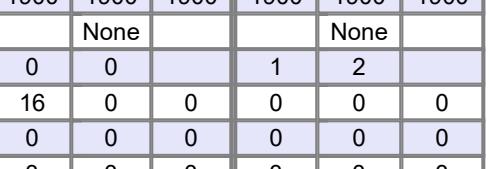
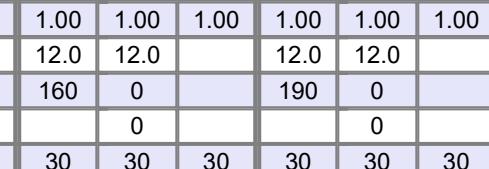
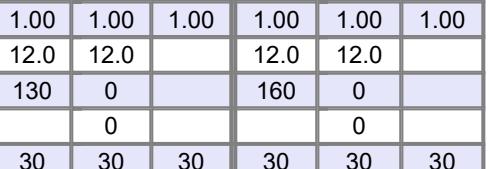
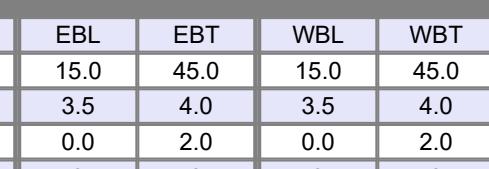
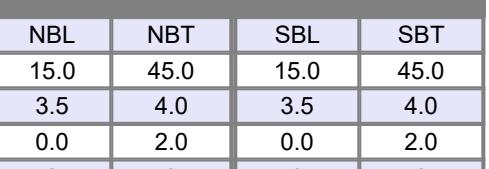
General Information						Intersection Information						
Agency	GHA			Duration, h			0.250					
Analyst	TM		Analysis Date	12/12/2019		Area Type		Other				
Jurisdiction	CCDOHT		Time Period	7:30-8:30 AM		PHF	0.93					
Urban Street	Landwehr Road		Analysis Year	2019		Analysis Period	1 > 7:30					
Intersection	Landwehr at Techny		File Name	AM Landwehr at Techny.xus								
Project Description	AM Peak											
Demand Information			EB		WB		NB		SB			
Approach Movement			L	T	R	L	T	R	L	T	R	
Demand (v), veh/h			50	283	73	135	126	76	60	387	194	
Signal Information												
Cycle, s	96.4	Reference Phase	2									
Offset, s	0	Reference Point	End	Green	3.4	3.3	38.3	3.2	0.8	24.9		
Uncoordinated	Yes	Simult. Gap E/W	On	Yellow	3.5	0.0	4.0	3.5	3.5	4.0		
Force Mode	Fixed	Simult. Gap N/S	On	Red	0.0	0.0	2.0	0.0	0.0	2.0		
Movement Group Results				EB		WB		NB		SB		
Approach Movement				L	T	R	L	T	R	L	T	R
Back of Queue (Q), ft/ln (95 th percentile)				40.9	351.4		107.2	187.5		37.8	538.6	
Back of Queue (Q), veh/ln (95 th percentile)				1.6	13.8		4.3	7.3		1.5	21.4	
Queue Storage Ratio (RQ) (95 th percentile)				0.26	0.00		0.56	0.00		0.29	0.00	
Control Delay (d), s/veh				25.5	38.8		26.0	27.6		16.8	38.3	
Level of Service (LOS)				C	D		C	C		B	D	
Approach Delay, s/veh / LOS				37.1		D	26.9		C	36.3		D
Intersection Delay, s/veh / LOS							31.0					C
												

--- Messages ---

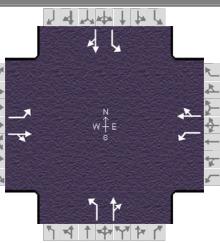
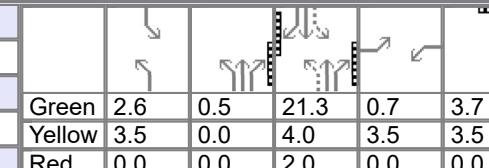
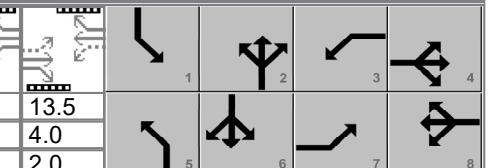
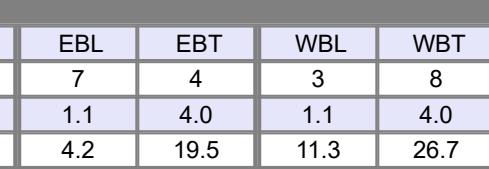
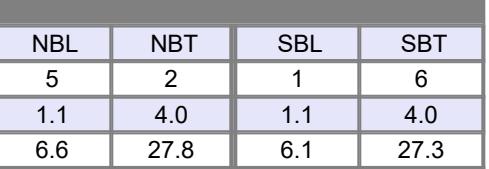
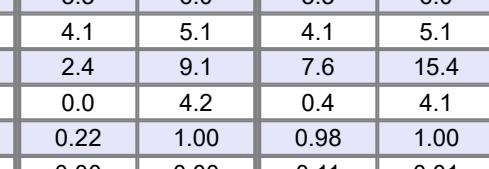
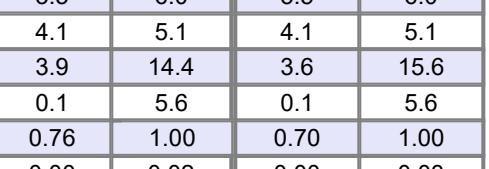
No errors or warnings exist.

--- Comments ---

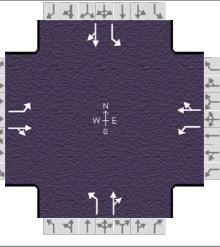
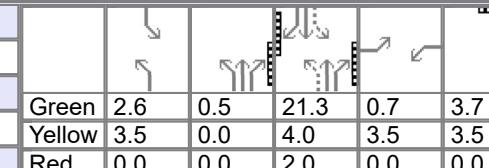
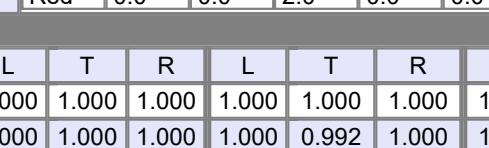
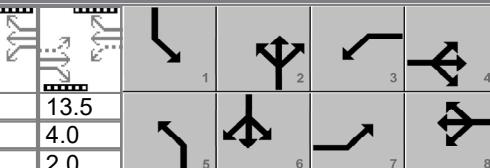
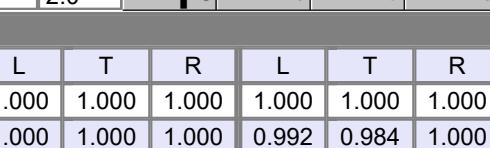
HCS7 Signalized Intersection Input Data

General Information							Intersection Information														
Agency	GHA			Duration, h	0.250																
Analyst	TM		Analysis Date	12/12/2019		Area Type	Other														
Jurisdiction	CCDOHT		Time Period	3:30-4:30 PM		PHF	0.96														
Urban Street	Landwehr Road		Analysis Year	2019		Analysis Period	1> 3:30														
Intersection	Landwehr at Techny		File Name	PM School Landwehr at Techny.xus																	
Project Description	PM School Peak																				
Demand Information				EB		WB		NB		SB											
Approach Movement				L	T	R	L	T	R	L	T	R									
Demand (v), veh/h				13	158	55	206	284	116	77	287	100									
										64	399	25									
Signal Information																					
Cycle, s	64.7	Reference Phase	2																		
Offset, s	0	Reference Point	End																		
Uncoordinated	Yes	Simult. Gap E/W	On																		
Force Mode	Fixed	Simult. Gap N/S	On																		
Traffic Information				EB		WB		NB		SB											
Approach Movement				L	T	R	L	T	R	L	T	R									
Demand (v), veh/h				13	158	55	206	284	116	77	287	100									
Initial Queue (Q _b), veh/h				0	0	0	0	0	0	0	0	0									
Base Saturation Flow Rate (s ₀), veh/h				1900	1900	1900	1900	1900	1900	1900	1900	1900									
Parking (N _m), man/h				None		None		None		None											
Heavy Vehicles (P _{HV}), %				0	0		0	1		0	0	1									
Ped / Bike / RTOR, /h				0	0	0	2	0	0	16	0	0									
Buses (N _b), buses/h				0	0	0	0	0	0	0	0	0									
Arrival Type (AT)				3	3	3	3	3	3	3	3	3									
Upstream Filtering (I)				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00									
Lane Width (W), ft				12.0	12.0		12.0	12.0		12.0	12.0	12.0									
Turn Bay Length, ft				160	0		190	0		130	0	160									
Grade (Pg), %					0			0			0										
Speed Limit, mi/h				30	30	30	30	30	30	30	30	30									
Phase Information				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT										
Maximum Green (G _{max}) or Phase Split, s				15.0	45.0	15.0	45.0	15.0	45.0	15.0	45.0										
Yellow Change Interval (Y), s				3.5	4.0	3.5	4.0	3.5	4.0	3.5	4.0										
Red Clearance Interval (R _c), s				0.0	2.0	0.0	2.0	0.0	2.0	0.0	2.0										
Minimum Green (G _{min}), s				3	8	3	8	3	8	3	8										
Start-Up Lost Time (It), s				2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0										
Extension of Effective Green (e), s				2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0										
Passage (PT), s				3.0	4.0	3.0	4.0	3.0	4.0	3.0	4.0										
Recall Mode				Off	Min	Off	Min	Off	Min	Off	Min										
Dual Entry				No	Yes	No	Yes	No	Yes	No	Yes										
Walk (Walk), s					7.0		7.0		7.0		7.0										
Pedestrian Clearance Time (PC), s					14.0		14.0		14.0		14.0										
Multimodal Information				EB		WB		NB		SB											
85th % Speed / Rest in Walk / Corner Radius				0	No	25	0	No	25	0	No	25									
Walkway / Crosswalk Width / Length, ft				9.0	12	0	9.0	12	0	9.0	12	0									
Street Width / Island / Curb				0	0	No	0	0	No	0	0	No									
Width Outside / Bike Lane / Shoulder, ft				12	5.0	2.0	12	5.0	2.0	12	5.0	2.0									
Pedestrian Signal / Occupied Parking				No	0.50		No	0.50		No	0.50										

HCS7 Signalized Intersection Results Summary

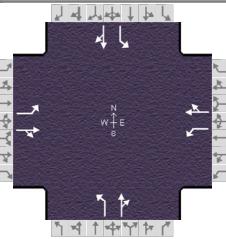
General Information						Intersection Information					
Agency	GHA			Duration, h			0.250				
Analyst	TM		Analysis Date	12/12/2019		Area Type		Other			
Jurisdiction	CCDOHT		Time Period	3:30-4:30 PM		PHF		0.96			
Urban Street	Landwehr Road		Analysis Year	2019		Analysis Period		1> 3:30			
Intersection	Landwehr at Techny		File Name	PM School Landwehr at Techny.xus							
Project Description	PM School Peak										
Demand Information			EB		WB		NB		SB		
Approach Movement			L	T	R	L	T	R	L		
Demand (v), veh/h			13	158	55	206	284	116	77		
									287		
									100		
									64		
									399		
									25		
Signal Information											
Cycle, s	64.7	Reference Phase	2								
Offset, s	0	Reference Point	End								
Uncoordinated	Yes	Simult. Gap E/W	On								
Force Mode	Fixed	Simult. Gap N/S	On								
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT		
Assigned Phase				7	4	3	8	5	2		
Case Number				1.1	4.0	1.1	4.0	1.1	4.0		
Phase Duration, s				4.2	19.5	11.3	26.7	6.6	27.8		
Change Period, (Y+R _c), s				3.5	6.0	3.5	6.0	3.5	6.0		
Max Allow Headway (MAH), s				4.1	5.1	4.1	5.1	4.1	5.1		
Queue Clearance Time (g _s), s				2.4	9.1	7.6	15.4	3.9	14.4		
Green Extension Time (g _e), s				0.0	4.2	0.4	4.1	0.1	5.6		
Phase Call Probability				0.22	1.00	0.98	1.00	0.76	1.00		
Max Out Probability				0.00	0.00	0.11	0.01	0.00	0.02		
Movement Group Results				EB		WB		NB		SB	
Approach Movement				L	T	R	L	T	R	L	
Assigned Movement				7	4	14	3	8	18	5	
Adjusted Flow Rate (v), veh/h				14	222		215	417		80	
Adjusted Saturation Flow Rate (s), veh/h/ln				1810	1816		1810	1790		1810	
Queue Service Time (g _s), s				0.4	7.1		5.6	13.4		1.9	
Cycle Queue Clearance Time (g _c), s				0.4	7.1		5.6	13.4		12.4	
Green Ratio (g/C)				0.22	0.21		0.36	0.32		0.38	
Capacity (c), veh/h				211	379		447	572		312	
Volume-to-Capacity Ratio (X)				0.064	0.586		0.480	0.728		0.664	
Back of Queue (Q), ft/ln (95 th percentile)				7.2	138.7		98.4	235		33	
Back of Queue (Q), veh/ln (95 th percentile)				0.3	5.5		3.9	9.3		8.7	
Queue Storage Ratio (RQ) (95 th percentile)				0.04	0.00		0.52	0.00		0.25	
Uniform Delay (d ₁), s/veh				20.4	23.1		15.8	19.5		14.7	
Incremental Delay (d ₂), s/veh				0.1	2.0		0.8	2.5		0.4	
Initial Queue Delay (d ₃), s/veh				0.0	0.0		0.0	0.0		0.0	
Control Delay (d), s/veh				20.6	25.2		16.6	22.1		15.1	
Level of Service (LOS)				C	C		B	C		B	
Approach Delay, s/veh / LOS				24.9	C		20.2	C		19.3	
Intersection Delay, s/veh / LOS				20.7				C			
Multimodal Results				EB		WB		NB		SB	
Pedestrian LOS Score / LOS				1.92	B		1.91	B		1.90	
Bicycle LOS Score / LOS				0.88	A		1.53	B		1.33	

HCS7 Signalized Intersection Intermediate Values

General Information							Intersection Information														
Agency	GHA			Duration, h	0.250																
Analyst	TM		Analysis Date	12/12/2019		Area Type	Other														
Jurisdiction	CCDOHT		Time Period	3:30-4:30 PM		PHF	0.96														
Urban Street	Landwehr Road		Analysis Year	2019		Analysis Period	1> 3:30														
Intersection	Landwehr at Techny		File Name	PM School Landwehr at Techny.xus																	
Project Description	PM School Peak																				
Demand Information				EB		WB		NB		SB											
Approach Movement				L	T	R	L	T	R	L	T	R									
Demand (v), veh/h				13	158	55	206	284	116	77	287	100									
Signal Information								EB			WB			NB			SB				
Cycle, s	64.7	Reference Phase	2					1	2	3	4										
Offset, s	0	Reference Point	End					Green	2.6	0.5	21.3	0.7	3.7	13.5							
Uncoordinated	Yes	Simult. Gap E/W	On					Yellow	3.5	0.0	4.0	3.5	3.5	4.0							
Force Mode	Fixed	Simult. Gap N/S	On					Red	0.0	0.0	2.0	0.0	0.0	2.0							
Saturation Flow / Delay				L	T	R	L	T	R	L	T	R	L	T	R						
Lane Width Adjustment Factor (f_w)				1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000						
Heavy Vehicles and Grade Factor (f_{Hvg})				1.000	1.000	1.000	1.000	0.992	1.000	1.000	1.000	1.000	0.992	0.984	1.000						
Parking Activity Adjustment Factor (f_p)				1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000						
Bus Blockage Adjustment Factor (f_{bb})				1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000						
Area Type Adjustment Factor (f_a)				1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000						
Lane Utilization Adjustment Factor (f_{LU})				1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000						
Left-Turn Adjustment Factor (f_{LT})				0.952	0.000		0.952	0.000		0.952	0.000		0.952	0.000							
Right-Turn Adjustment Factor (f_{RT})					0.956	0.956		0.949	0.949		0.949	0.949		0.989	0.989						
Left-Turn Pedestrian Adjustment Factor (f_{Lpb})				0.998			1.000			1.000			0.991								
Right-Turn Ped-Bike Adjustment Factor (f_{Rpb})						1.000			0.997			0.976			1.000						
Work Zone Adjustment Factor (f_{wz})				1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000						
DDI Factor (f_{DDI})				1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000						
Movement Saturation Flow Rate (s), veh/h				1810	1347	469	1810	1271	519	1810	1337	466	1795	1742	109						
Proportion of Vehicles Arriving on Green (P)				0.01	0.21	0.21	0.12	0.32	0.32	0.05	0.34	0.34	0.04	0.33	0.33						
Incremental Delay Factor (k)				0.11	0.15		0.11	0.15		0.11	0.15		0.11	0.15							
Signal Timing / Movement Groups				EBL	EBT/R	WBL	WBT/R	NBL	NBT/R	SBL	SBT/R										
Lost Time (t_L)				3.5	6.0	3.5	6.0	3.5	6.0	3.5	6.0										
Green Ratio (g/C)				0.22	0.21	0.36	0.32	0.38	0.34	0.37	0.33										
Permitted Saturation Flow Rate (s_p), veh/h/ln				985	0	1177	0	963	0	990	0										
Shared Saturation Flow Rate (s_{sh}), veh/h/ln																					
Permitted Effective Green Time (g_p), s				13.5	0.0	15.5	0.0	21.3	0.0	21.3	0.0										
Permitted Service Time (g_u), s				5.4	0.0	6.4	0.0	7.7	0.0	7.5	0.0										
Permitted Queue Service Time (g_{ps}), s				0.1		2.0		1.2		1.0											
Time to First Blockage (g_f), s				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0										
Queue Service Time Before Blockage (g_{fs}), s																					
Protected Right Saturation Flow (s_R), veh/h/ln																					
Protected Right Effective Green Time (g_R), s																					
Multimodal				EB		WB		NB		SB											
Pedestrian F_w / F_v				1.198	0.000	1.198	0.000	1.198	0.000	1.198	0.000										
Pedestrian F_s / F_{delay}				0.000	0.121	0.000	0.109	0.000	0.106	0.000	0.107										
Pedestrian M_{corner} / M_{cw}																					
Bicycle c_b / d_b				416.96	20.26	639.10	14.97	673.32	14.23	658.15	14.56										
Bicycle F_w / F_v				-3.64	0.39	-3.64	1.04	-3.64	0.80	-3.64	0.84										

HCS7 Signalized Intersection Results Graphical Summary

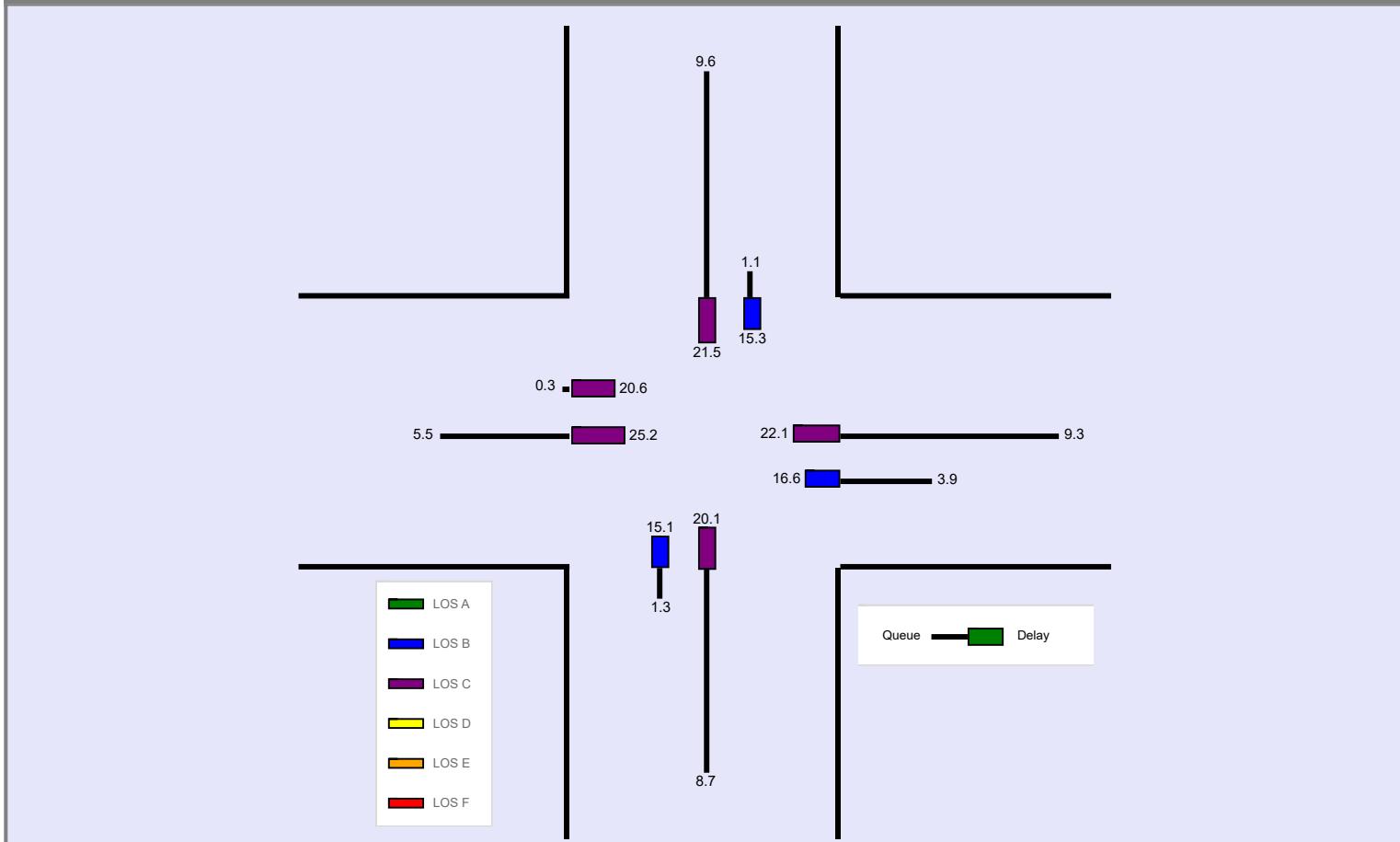
General Information				Intersection Information	
Agency	GHA		Duration, h	0.250	
Analyst	TM	Analysis Date	12/12/2019	Area Type	Other
Jurisdiction	CDDOT	Time Period	3:30-4:30 PM	PHF	0.96
Urban Street	Landwehr Road	Analysis Year	2019	Analysis Period	1 > 3:30
Intersection	Landwehr at Techny	File Name	PM School Landwehr at Techny.xus		
Project Description	PM School Peak				



Demand Information			EB			WB			NB			SB		
Approach Movement			L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h			13	158	55	206	284	116	77	287	100	64	399	25

Signal Information				Phase Sequence											
Cycle, s	64.7	Reference Phase	2	1	2	3	4	5	6	7	8	1	2	3	4
Offset, s	0	Reference Point	End	Green	2.6	0.5	21.3	0.7	3.7	13.5		1	2	3	4
Uncoordinated	Yes	Simult. Gap E/W	On	Yellow	3.5	0.0	4.0	3.5	3.5	4.0					
Force Mode	Fixed	Simult. Gap N/S	On	Red	0.0	0.0	2.0	0.0	0.0	2.0		5	6	7	8

Movement Group Results			EB			WB			NB			SB		
Approach Movement			L	T	R	L	T	R	L	T	R	L	T	R
Back of Queue (Q), ft/ln (95 th percentile)			7.2	138.7		98.4	235		33	216.4		27.9	245	
Back of Queue (Q), veh/ln (95 th percentile)			0.3	5.5		3.9	9.3		1.3	8.7		1.1	9.6	
Queue Storage Ratio (RQ) (95 th percentile)			0.04	0.00		0.52	0.00		0.25	0.00		0.17	0.00	
Control Delay (d), s/veh			20.6	25.2		16.6	22.1		15.1	20.1		15.3	21.5	
Level of Service (LOS)			C	C		B	C		B	C		B	C	
Approach Delay, s/veh / LOS			24.9	C		20.2	C		19.3	B		20.7	C	
Intersection Delay, s/veh / LOS						20.7						C		

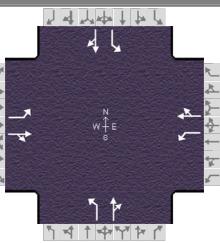
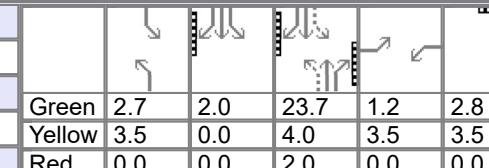
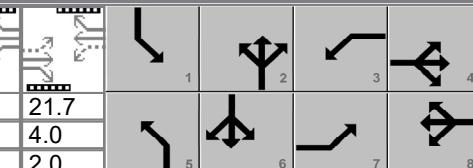


--- Messages ---

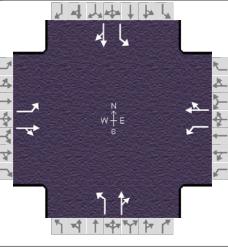
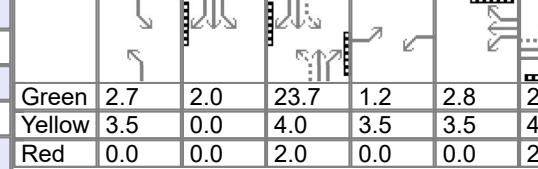
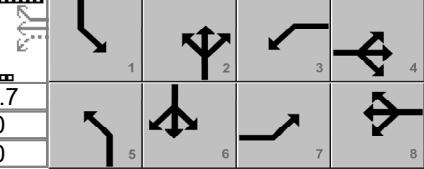
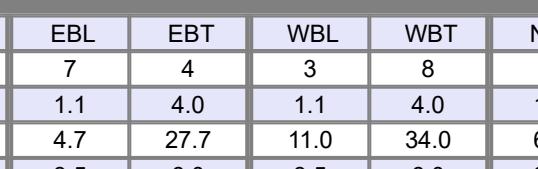
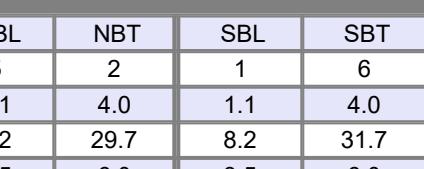
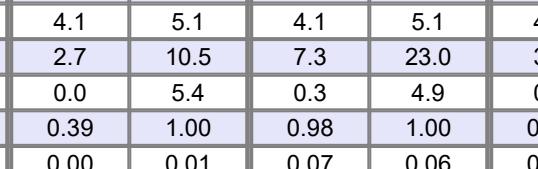
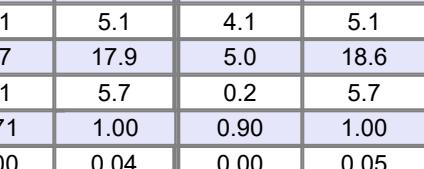
No errors or warnings exist.

--- Comments ---

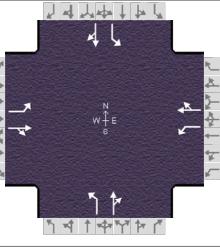
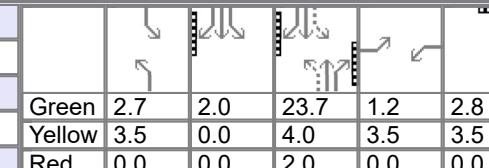
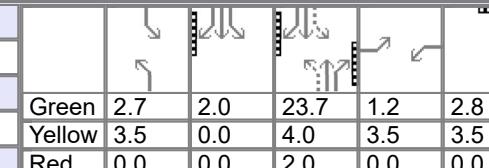
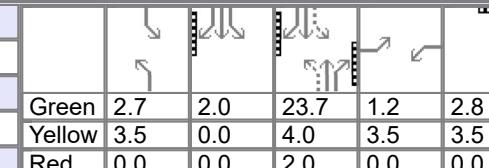
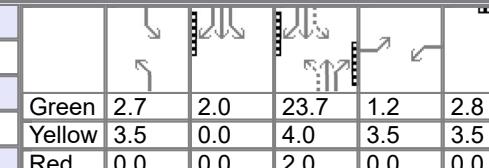
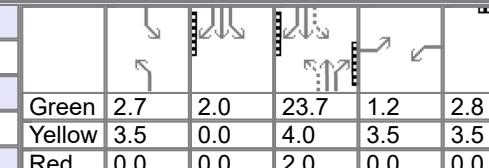
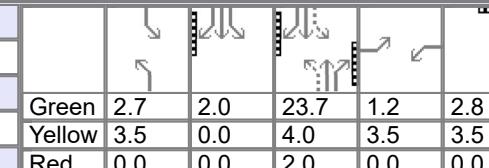
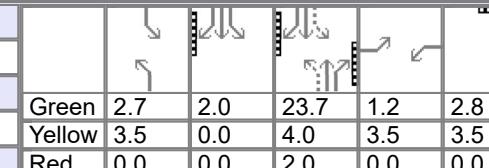
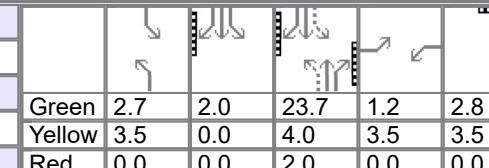
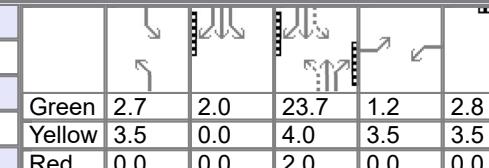
HCS7 Signalized Intersection Input Data

General Information						Intersection Information													
Agency	GHA					Duration, h	0.250												
Analyst	TM		Analysis Date	12/12/2019		Area Type	Other												
Jurisdiction	CCDOHT		Time Period	5:00-6:00 PM		PHF	0.96												
Urban Street	Landwehr Road		Analysis Year	2019		Analysis Period	1> 5:00												
Intersection	Landwehr at Techny		File Name	PM Street Landwehr at Techny.xus															
Project Description	PM Street Peak																		
Demand Information				EB		WB		NB		SB									
Approach Movement				L	T	R	L	T	R	L	T	R							
Demand (v), veh/h				22	170	62	180	382	138	56	312	92							
										103	400	35							
Signal Information																			
Cycle, s	76.5	Reference Phase	2	Green	2.7	2.0	23.7	1.2	2.8	21.7									
Offset, s	0	Reference Point	End	Yellow	3.5	0.0	4.0	3.5	3.5	4.0									
Uncoordinated	Yes	Simult. Gap E/W	On	Red	0.0	0.0	2.0	0.0	0.0	2.0									
Force Mode	Fixed	Simult. Gap N/S	On																
Traffic Information				EB		WB		NB		SB									
Approach Movement				L	T	R	L	T	R	L	T	R							
Demand (v), veh/h				22	170	62	180	382	138	56	312	92							
Initial Queue (Q _b), veh/h				0	0	0	0	0	0	0	0	0							
Base Saturation Flow Rate (s ₀), veh/h				1900	1900	1900	1900	1900	1900	1900	1900	1900							
Parking (N _m), man/h					None			None			None								
Heavy Vehicles (P _{HV}), %				0	0		0	1		0	0	1							
Ped / Bike / RTOR, /h				0	0	0	0	0	0	1	0	0							
Buses (N _b), buses/h				0	0	0	0	0	0	0	0	0							
Arrival Type (AT)				3	3	3	3	3	3	3	3	3							
Upstream Filtering (I)				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00							
Lane Width (W), ft				12.0	12.0		12.0	12.0		12.0	12.0	12.0							
Turn Bay Length, ft				160	0		190	0		130	0	160							
Grade (Pg), %					0			0			0								
Speed Limit, mi/h				30	30	30	30	30	30	30	30	30							
Phase Information				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT								
Maximum Green (G _{max}) or Phase Split, s				15.0	45.0	15.0	45.0	15.0	45.0	15.0	45.0								
Yellow Change Interval (Y), s				3.5	4.0	3.5	4.0	3.5	4.0	3.5	4.0								
Red Clearance Interval (R _c), s				0.0	2.0	0.0	2.0	0.0	2.0	0.0	2.0								
Minimum Green (G _{min}), s				3	8	3	8	3	8	3	8								
Start-Up Lost Time (It), s				2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0								
Extension of Effective Green (e), s				2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0								
Passage (PT), s				3.0	4.0	3.0	4.0	3.0	4.0	3.0	4.0								
Recall Mode				Off	Min	Off	Min	Off	Min	Off	Min								
Dual Entry				No	Yes	No	Yes	No	Yes	No	Yes								
Walk (Walk), s					7.0		7.0		7.0		7.0								
Pedestrian Clearance Time (PC), s					14.0		14.0		14.0		14.0								
Multimodal Information				EB		WB		NB		SB									
85th % Speed / Rest in Walk / Corner Radius				0	No	25	0	No	25	0	No	25							
Walkway / Crosswalk Width / Length, ft				9.0	12	0	9.0	12	0	9.0	12	0							
Street Width / Island / Curb				0	0	No	0	0	No	0	0	No							
Width Outside / Bike Lane / Shoulder, ft				12	5.0	2.0	12	5.0	2.0	12	5.0	2.0							
Pedestrian Signal / Occupied Parking				No	0.50		No	0.50		No	0.50								

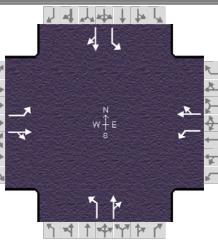
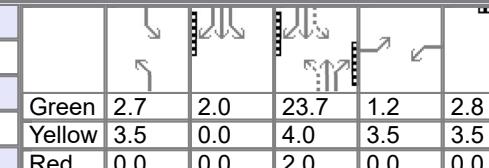
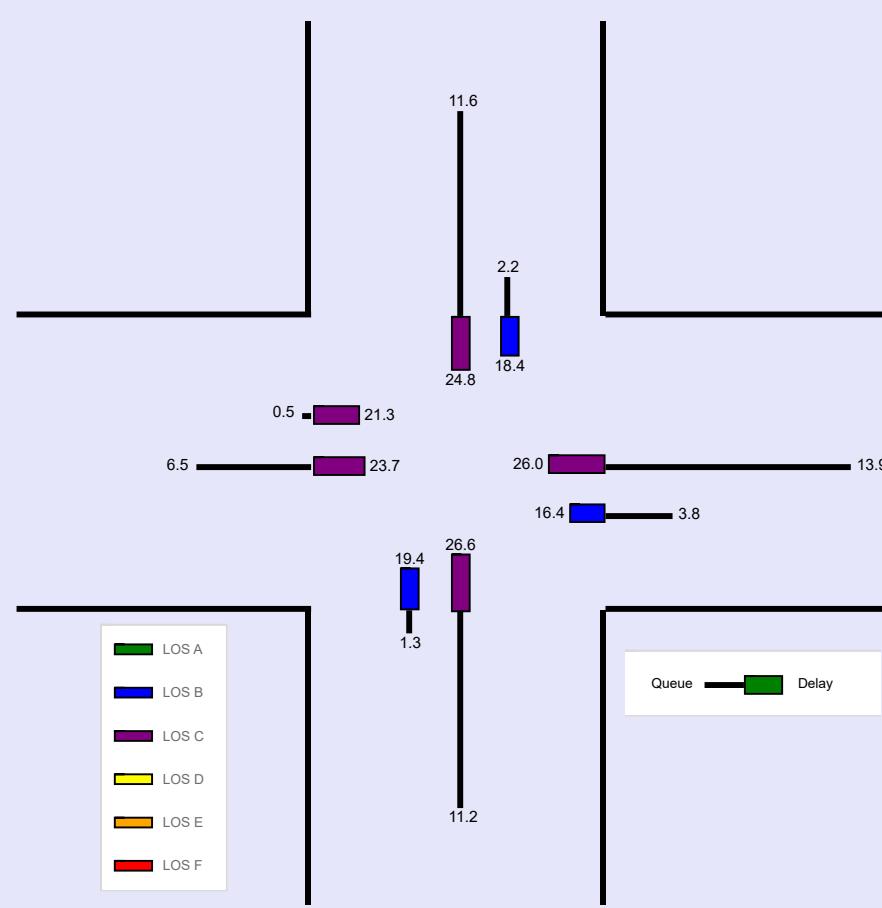
HCS7 Signalized Intersection Results Summary

General Information						Intersection Information						
Agency	GHA			Duration, h			0.250					
Analyst	TM		Analysis Date	12/12/2019		Area Type		Other				
Jurisdiction	CCDOHT		Time Period	5:00-6:00 PM		PHF		0.96				
Urban Street	Landwehr Road		Analysis Year	2019		Analysis Period		1> 5:00				
Intersection	Landwehr at Techny		File Name	PM Street Landwehr at Techny.xus								
Project Description	PM Street Peak											
Demand Information			EB		WB		NB		SB			
Approach Movement			L	T	R	L	T	R	L			
Demand (v), veh/h			22	170	62	180	382	138	56			
									T			
									R			
Signal Information												
Cycle, s	76.5	Reference Phase	2									
Offset, s	0	Reference Point	End		Green	2.7	2.0	23.7	1.2			
Uncoordinated	Yes	Simult. Gap E/W	On		Yellow	3.5	0.0	4.0	3.5			
Force Mode	Fixed	Simult. Gap N/S	On		Red	0.0	0.0	2.0	0.0			
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	
Assigned Phase				7	4	3	8	5	2	1	6	
Case Number				1.1	4.0	1.1	4.0	1.1	4.0	1.1	4.0	
Phase Duration, s				4.7	27.7	11.0	34.0	6.2	29.7	8.2	31.7	
Change Period, (Y+R _c), s				3.5	6.0	3.5	6.0	3.5	6.0	3.5	6.0	
Max Allow Headway (MAH), s				4.1	5.1	4.1	5.1	4.1	5.1	4.1	5.1	
Queue Clearance Time (g _s), s				2.7	10.5	7.3	23.0	3.7	17.9	5.0	18.6	
Green Extension Time (g _e), s				0.0	5.4	0.3	4.9	0.1	5.7	0.2	5.7	
Phase Call Probability				0.39	1.00	0.98	1.00	0.71	1.00	0.90	1.00	
Max Out Probability				0.00	0.01	0.07	0.06	0.00	0.04	0.00	0.05	
Movement Group Results				EB		WB		NB		SB		
Approach Movement				L	T	R	L	T	R	L	T	R
Assigned Movement				7	4	14	3	8	18	5	2	12
Adjusted Flow Rate (v), veh/h				23	242		188	542		58	421	
Adjusted Saturation Flow Rate (s), veh/h/ln				1810	1813		1810	1799		1810	1824	
Queue Service Time (g _s), s				0.7	8.5		5.3	21.0		1.7	15.9	
Cycle Queue Clearance Time (g _c), s				0.7	8.5		5.3	21.0		1.7	15.9	
Green Ratio (g/C)				0.30	0.28		0.41	0.37		0.35	0.31	
Capacity (c), veh/h				180	513		472	658		247	565	
Volume-to-Capacity Ratio (X)				0.127	0.471		0.397	0.823		0.236	0.745	
Back of Queue (Q), ft/ln (95 th percentile)				13.3	161.7		94.6	349.3		31.5	280.3	
Back of Queue (Q), veh/ln (95 th percentile)				0.5	6.5		3.8	13.9		1.3	11.2	
Queue Storage Ratio (RQ) (95 th percentile)				0.08	0.00		0.50	0.00		0.24	0.00	
Uniform Delay (d ₁), s/veh				21.0	22.8		15.9	22.1		18.9	23.8	
Incremental Delay (d ₂), s/veh				0.3	1.0		0.5	4.0		0.5	2.8	
Initial Queue Delay (d ₃), s/veh				0.0	0.0		0.0	0.0		0.0	0.0	
Control Delay (d), s/veh				21.3	23.7		16.4	26.0		19.4	26.6	
Level of Service (LOS)				C	C		B	C		B	C	
Approach Delay, s/veh / LOS				23.5	C		23.6	C		25.7	C	
Intersection Delay, s/veh / LOS				24.1				C				
Multimodal Results				EB		WB		NB		SB		
Pedestrian LOS Score / LOS				1.92	B		1.91	B		1.91	B	
Bicycle LOS Score / LOS				0.92	A		1.69	B		1.28	A	

HCS7 Signalized Intersection Intermediate Values

General Information							Intersection Information														
Agency	GHA			Duration, h	0.250																
Analyst	TM		Analysis Date	12/12/2019		Area Type	Other														
Jurisdiction	CCDOHT		Time Period	5:00-6:00 PM		PHF	0.96														
Urban Street	Landwehr Road		Analysis Year	2019		Analysis Period	1> 5:00														
Intersection	Landwehr at Techny		File Name	PM Street Landwehr at Techny.xus																	
Project Description	PM Street Peak																				
Demand Information				EB		WB		NB		SB											
Approach Movement				L	T	R	L	T	R	L	T	R									
Demand (v), veh/h				22	170	62	180	382	138	56	312	92									
				103	400	35															
Signal Information																					
Cycle, s	76.5	Reference Phase	2																		
Offset, s	0	Reference Point	End		Green	2.7	2.0	23.7	1.2	2.8	21.7										
Uncoordinated	Yes	Simult. Gap E/W	On		Yellow	3.5	0.0	4.0	3.5	3.5	4.0										
Force Mode	Fixed	Simult. Gap N/S	On		Red	0.0	0.0	2.0	0.0	0.0	2.0										
Saturation Flow / Delay				L	T	R	L	T	R	L	T	R									
Lane Width Adjustment Factor (f_w)				1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000									
Heavy Vehicles and Grade Factor (f_{Hvg})				1.000	1.000	1.000	1.000	0.992	1.000	1.000	1.000	0.992									
Parking Activity Adjustment Factor (f_p)				1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000									
Bus Blockage Adjustment Factor (f_{bb})				1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000									
Area Type Adjustment Factor (f_a)				1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000									
Lane Utilization Adjustment Factor (f_{LU})				1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000									
Left-Turn Adjustment Factor (f_{LT})				0.952	0.000		0.952	0.000		0.952	0.000										
Right-Turn Adjustment Factor (f_{RT})					0.954	0.954		0.954	0.954		0.960	0.960									
Left-Turn Pedestrian Adjustment Factor (f_{Lpb})				1.000			1.000			1.000											
Right-Turn Ped-Bike Adjustment Factor (f_{Rpb})						1.000			1.000		0.998										
Work Zone Adjustment Factor (f_{wz})				1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000									
DDI Factor (f_{DDI})				1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000									
Movement Saturation Flow Rate (s), veh/h				1810	1328	484	1810	1322	477	1810	1409	415									
Proportion of Vehicles Arriving on Green (P)				0.02	0.28	0.28	0.10	0.37	0.37	0.04	0.31	0.31									
Incremental Delay Factor (k)				0.11	0.15		0.11	0.16		0.11	0.15										
Signal Timing / Movement Groups				EBL	EBT/R	WBL	WBT/R	NBL	NBT/R	SBL	SBT/R										
Lost Time (t_L)				3.5	6.0	3.5	6.0	3.5	6.0	3.5	6.0										
Green Ratio (g/C)				0.30	0.28	0.41	0.37	0.35	0.31	0.38	0.34										
Permitted Saturation Flow Rate (s_p), veh/h/ln				878	0	1156	0	953	0	974	0										
Shared Saturation Flow Rate (s_{sh}), veh/h/ln																					
Permitted Effective Green Time (g_p), s				21.7	0.0	23.7	0.0	23.8	0.0	24.3	0.0										
Permitted Service Time (g_u), s				5.1	0.0	13.3	0.0	7.2	0.0	7.9	0.0										
Permitted Queue Service Time (g_{ps}), s				0.4		2.0		1.1		2.0											
Time to First Blockage (g_f), s				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0										
Queue Service Time Before Blockage (g_{fs}), s																					
Protected Right Saturation Flow (s_R), veh/h/ln																					
Protected Right Effective Green Time (g_R), s																					
Multimodal				EB		WB		NB		SB											
Pedestrian F_w / F_v				1.198	0.000	1.198	0.000	1.198	0.000	1.198	0.000										
Pedestrian F_s / F_{delay}				0.000	0.119	0.000	0.110	0.000	0.116	0.000	0.113										
Pedestrian M_{corner} / M_{cw}																					
Bicycle c_b / d_b				565.86	19.68	731.12	15.41	619.03	18.25	671.82	16.88										
Bicycle F_w / F_v				-3.64	0.44	-3.64	1.20	-3.64	0.79	-3.64	0.92										

HCS7 Signalized Intersection Results Graphical Summary

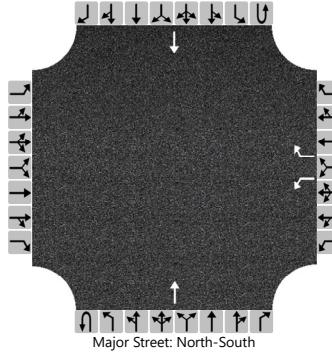
General Information						Intersection Information					
Agency	GHA			Duration, h			0.250				
Analyst	TM		Analysis Date	12/12/2019		Area Type		Other			
Jurisdiction	CCDOHT		Time Period	5:00-6:00 PM		PHF		0.96			
Urban Street	Landwehr Road		Analysis Year	2019		Analysis Period		1 > 5:00			
Intersection	Landwehr at Techny		File Name	PM Street Landwehr at Techny.xus							
Project Description	PM Street Peak										
Demand Information			EB		WB		NB		SB		
Approach Movement			L	T	R	L	T	R	L		
Demand (v), veh/h			22	170	62	180	382	138	56		
									T		
									R		
Signal Information											
Cycle, s	76.5	Reference Phase	2	2.7	2.0	23.7	1.2	2.8			
Offset, s	0	Reference Point	End	Green	2.7	2.0	23.7	1.2	21.7		
Uncoordinated	Yes	Simult. Gap E/W	On	Yellow	3.5	0.0	4.0	3.5	3.5		
Force Mode	Fixed	Simult. Gap N/S	On	Red	0.0	0.0	2.0	0.0	2.0		
Movement Group Results				EB		WB		NB		SB	
Approach Movement				L	T	R	L	T	R	L	
Back of Queue (Q), ft/ln (95 th percentile)				13.3	161.7		94.6	349.3		31.5	
Back of Queue (Q), veh/ln (95 th percentile)				0.5	6.5		3.8	13.9		11.2	
Queue Storage Ratio (RQ) (95 th percentile)				0.08	0.00		0.50	0.00		0.24	
Control Delay (d), s/veh				21.3	23.7		16.4	26.0		19.4	
Level of Service (LOS)				C	C		B	C		B	
Approach Delay, s/veh / LOS				23.5	C		23.6	C		23.6	
Intersection Delay, s/veh / LOS							24.1			C	
											

--- Messages ---

No errors or warnings exist.

--- Comments ---

HCS7 Two-Way Stop-Control Report

General Information				Site Information																										
Analyst	TM			Intersection		Landwehr at N School																								
Agency/Co.	GHA			Jurisdiction		CCDOH																								
Date Performed	12/12/2019			East/West Street		North School Access																								
Analysis Year	2019			North/South Street		Landwehr Road																								
Time Analyzed	7:30-8:30 AM			Peak Hour Factor		0.89																								
Intersection Orientation	North-South			Analysis Time Period (hrs)		0.25																								
Project Description	Existing AM																													
Lanes																														
 Major Street: North-South																														
Vehicle Volumes and Adjustments																														
Approach	Eastbound			Westbound			Northbound			Southbound																				
Movement	U	L	T	R	U	L	T	R	U	L	T	R																		
Priority		10	11	12		7	8	9	1U	1	2	3																		
Number of Lanes		0	0	0		1	0	1	0	0	0	1																		
Configuration					L		R		T			T																		
Volume (veh/h)					51		39		602			470																		
Percent Heavy Vehicles (%)					0		0																							
Proportion Time Blocked																														
Percent Grade (%)					0																									
Right Turn Channelized					No																									
Median Type Storage	Undivided																													
Critical and Follow-up Headways																														
Base Critical Headway (sec)					7.1		6.2																							
Critical Headway (sec)					6.40		6.20																							
Base Follow-Up Headway (sec)					3.5		3.3																							
Follow-Up Headway (sec)					3.50		3.30																							
Delay, Queue Length, and Level of Service																														
Flow Rate, v (veh/h)					57		44																							
Capacity, c (veh/h)					204		454																							
v/c Ratio					0.28		0.10																							
95% Queue Length, Q ₉₅ (veh)					1.1		0.3																							
Control Delay (s/veh)					29.5		13.8																							
Level of Service (LOS)					D		B																							
Approach Delay (s/veh)				22.7																										
Approach LOS				C																										

Pedestrian Level of Service

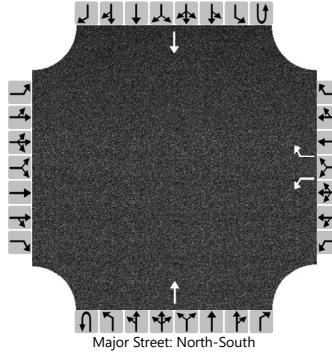
Flow (ped/hr)		3	0	0
Two-Stage Crossing		No		
Pedestrian Platooning		No		
Conflicting Vehicular Flow (veh/h)				
Average Delay (s)		0.4		
Level of Service (LOS)		A		

Copyright © 2019 University of Florida. All Rights Reserved.

HCS™ TWSC Version 7.8.5
Existing AM Landwehr at N School.xtw

Generated: 12/12/2019 10:50:39 AM

HCS7 Two-Way Stop-Control Report

General Information				Site Information																										
Analyst	TM			Intersection		Landwehr at N School																								
Agency/Co.	GHA			Jurisdiction		CCDOHT																								
Date Performed	12/12/2019			East/West Street		North School Access																								
Analysis Year	2019			North/South Street		Landwehr Road																								
Time Analyzed	3:30-4:30 PM			Peak Hour Factor		0.92																								
Intersection Orientation	North-South			Analysis Time Period (hrs)		0.25																								
Project Description	Existing PM School																													
Lanes																														
 Major Street: North-South																														
Vehicle Volumes and Adjustments																														
Approach	Eastbound			Westbound			Northbound			Southbound																				
Movement	U	L	T	R	U	L	T	R	U	L	T	R																		
Priority		10	11	12		7	8	9	1U	1	2	3																		
Number of Lanes		0	0	0		1	0	1	0	0	0	1																		
Configuration					L		R		T			T																		
Volume (veh/h)					31		22		442			660																		
Percent Heavy Vehicles (%)					0		0																							
Proportion Time Blocked																														
Percent Grade (%)				0																										
Right Turn Channelized				No																										
Median Type Storage	Undivided																													
Critical and Follow-up Headways																														
Base Critical Headway (sec)					7.1		6.2																							
Critical Headway (sec)					6.40		6.20																							
Base Follow-Up Headway (sec)					3.5		3.3																							
Follow-Up Headway (sec)					3.50		3.30																							
Delay, Queue Length, and Level of Service																														
Flow Rate, v (veh/h)					34		24																							
Capacity, c (veh/h)					206		586																							
v/c Ratio					0.16		0.04																							
95% Queue Length, Q ₉₅ (veh)					0.6		0.1																							
Control Delay (s/veh)					25.9		11.4																							
Level of Service (LOS)					D		B																							
Approach Delay (s/veh)				19.9																										
Approach LOS				C																										

Pedestrian Level of Service

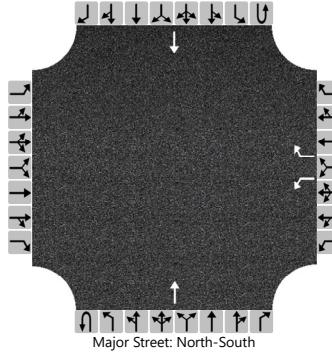
Flow (ped/hr)		3	0	0
Two-Stage Crossing		No		
Pedestrian Platooning		No		
Conflicting Vehicular Flow (veh/h)				
Average Delay (s)		0.2		
Level of Service (LOS)		A		

Copyright © 2019 University of Florida. All Rights Reserved.

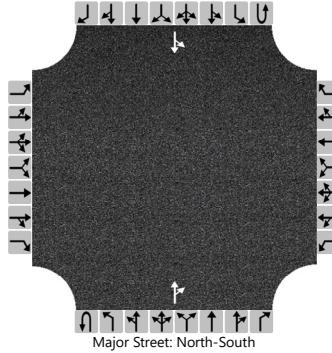
HCS™ TWSC Version 7.8.5
Existing PM School Landwehr at N School.xtw

Generated: 12/12/2019 10:52:29 AM

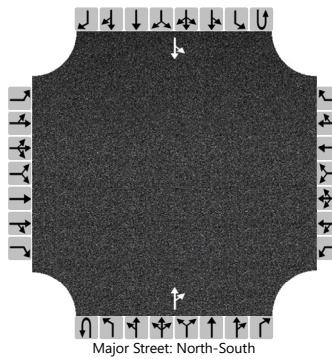
HCS7 Two-Way Stop-Control Report

General Information				Site Information																										
Analyst	TM			Intersection		Landwehr at N School																								
Agency/Co.	GHA			Jurisdiction		CCDOH																								
Date Performed	12/12/2019			East/West Street		North School Access																								
Analysis Year	2019			North/South Street		Landwehr Road																								
Time Analyzed	5:00-6:00 PM			Peak Hour Factor		0.92																								
Intersection Orientation	North-South			Analysis Time Period (hrs)		0.25																								
Project Description	Existing PM School																													
Lanes																														
 Major Street: North-South																														
Vehicle Volumes and Adjustments																														
Approach	Eastbound			Westbound			Northbound			Southbound																				
Movement	U	L	T	U	L	T	U	L	T	U	L	T																		
Priority		10	11	12		7	8	9	1U	1	2	3																		
Number of Lanes		0	0	0		1	0	1	0	0	0	1																		
Configuration					L		R		T			T																		
Volume (veh/h)					3		6		454			642																		
Percent Heavy Vehicles (%)					0		0																							
Proportion Time Blocked																														
Percent Grade (%)					0																									
Right Turn Channelized					No																									
Median Type Storage	Undivided																													
Critical and Follow-up Headways																														
Base Critical Headway (sec)						7.1		6.2																						
Critical Headway (sec)						6.40		6.20																						
Base Follow-Up Headway (sec)						3.5		3.3																						
Follow-Up Headway (sec)						3.50		3.30																						
Delay, Queue Length, and Level of Service																														
Flow Rate, v (veh/h)					3		7																							
Capacity, c (veh/h)					209		580																							
v/c Ratio					0.02		0.01																							
95% Queue Length, Q ₉₅ (veh)					0.0		0.0																							
Control Delay (s/veh)					22.5		11.3																							
Level of Service (LOS)					C		B																							
Approach Delay (s/veh)				15.0																										
Approach LOS				C																										

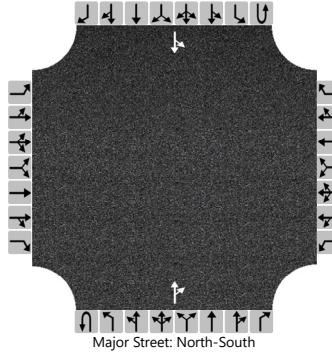
HCS7 Two-Way Stop-Control Report

General Information				Site Information																										
Analyst	TM			Intersection		Landwehr at S School																								
Agency/Co.	GHA			Jurisdiction		CCDOH																								
Date Performed	12/12/2019			East/West Street		South School Access																								
Analysis Year	2019			North/South Street		Landwehr Road																								
Time Analyzed	7:30-8:30 AM			Peak Hour Factor		0.88																								
Intersection Orientation	North-South			Analysis Time Period (hrs)		0.25																								
Project Description	Existing AM																													
Lanes																														
 Major Street: North-South																														
Vehicle Volumes and Adjustments																														
Approach	Eastbound			Westbound			Northbound			Southbound																				
Movement	U	L	T	R	U	L	T	R	U	L	T	R																		
Priority		10	11	12		7	8	9	1U	1	2	3																		
Number of Lanes		0	0	0		0	0	0	0	0	0	1																		
Configuration									TR		LT																			
Volume (veh/h)									602	78		16																		
Percent Heavy Vehicles (%)												0																		
Proportion Time Blocked																														
Percent Grade (%)																														
Right Turn Channelized																														
Median Type Storage	Undivided																													
Critical and Follow-up Headways																														
Base Critical Headway (sec)										4.1																				
Critical Headway (sec)										4.10																				
Base Follow-Up Headway (sec)										2.2																				
Follow-Up Headway (sec)										2.20																				
Delay, Queue Length, and Level of Service																														
Flow Rate, v (veh/h)										18																				
Capacity, c (veh/h)										852																				
v/c Ratio										0.02																				
95% Queue Length, Q ₉₅ (veh)										0.1																				
Control Delay (s/veh)										9.3																				
Level of Service (LOS)										A																				
Approach Delay (s/veh)										0.6																				
Approach LOS																														

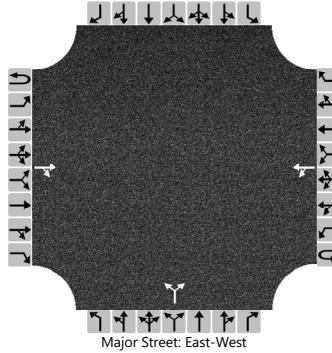
HCS7 Two-Way Stop-Control Report

General Information				Site Information																										
Analyst	TM			Intersection		Landwehr at S School																								
Agency/Co.	GHA			Jurisdiction		CCDOH																								
Date Performed	12/12/2019			East/West Street		South School Access																								
Analysis Year	2019			North/South Street		Landwehr Road																								
Time Analyzed	3:30-4:30 PM			Peak Hour Factor		0.93																								
Intersection Orientation	North-South			Analysis Time Period (hrs)		0.25																								
Project Description	Existing PM School																													
Lanes																														
 Major Street: North-South																														
Vehicle Volumes and Adjustments																														
Approach	Eastbound			Westbound			Northbound			Southbound																				
Movement	U	L	T	R	U	L	T	R	U	L	T	R																		
Priority		10	11	12		7	8	9	1U	1	2	3																		
Number of Lanes		0	0	0		0	0	0	0	0	0	1																		
Configuration										TR		LT																		
Volume (veh/h)									442	22		5																		
Percent Heavy Vehicles (%)												0																		
Proportion Time Blocked																														
Percent Grade (%)																														
Right Turn Channelized																														
Median Type Storage	Undivided																													
Critical and Follow-up Headways																														
Base Critical Headway (sec)										4.1																				
Critical Headway (sec)										4.10																				
Base Follow-Up Headway (sec)										2.2																				
Follow-Up Headway (sec)										2.20																				
Delay, Queue Length, and Level of Service																														
Flow Rate, v (veh/h)										5																				
Capacity, c (veh/h)										1076																				
v/c Ratio										0.00																				
95% Queue Length, Q ₉₅ (veh)										0.0																				
Control Delay (s/veh)										8.4																				
Level of Service (LOS)										A																				
Approach Delay (s/veh)										0.1																				
Approach LOS																														

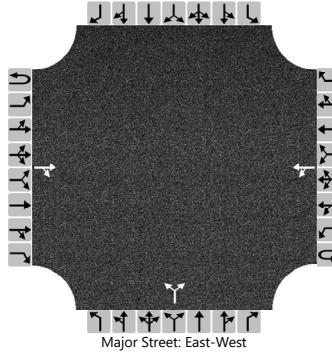
HCS7 Two-Way Stop-Control Report

General Information				Site Information																										
Analyst	TM			Intersection		Landwehr at S School																								
Agency/Co.	GHA			Jurisdiction		CCDOH																								
Date Performed	12/12/2019			East/West Street		South School Access																								
Analysis Year	2019			North/South Street		Landwehr Road																								
Time Analyzed	5:00-6:00 PM			Peak Hour Factor		0.93																								
Intersection Orientation	North-South			Analysis Time Period (hrs)		0.25																								
Project Description	Existing PM School																													
Lanes																														
 <p>Major Street: North-South</p>																														
Vehicle Volumes and Adjustments																														
Approach	Eastbound			Westbound			Northbound			Southbound																				
Movement	U	L	T	R	U	L	T	R	U	L	T	R																		
Priority		10	11	12		7	8	9	1U	1	2	3																		
Number of Lanes		0	0	0		0	0	0	0	0	0	1																		
Configuration										TR		LT																		
Volume (veh/h)									454	2		3																		
Percent Heavy Vehicles (%)												0																		
Proportion Time Blocked																														
Percent Grade (%)																														
Right Turn Channelized																														
Median Type Storage	Undivided																													
Critical and Follow-up Headways																														
Base Critical Headway (sec)										4.1																				
Critical Headway (sec)										4.10																				
Base Follow-Up Headway (sec)										2.2																				
Follow-Up Headway (sec)										2.20																				
Delay, Queue Length, and Level of Service																														
Flow Rate, v (veh/h)										3																				
Capacity, c (veh/h)										1083																				
v/c Ratio										0.00																				
95% Queue Length, Q ₉₅ (veh)										0.0																				
Control Delay (s/veh)										8.3																				
Level of Service (LOS)										A																				
Approach Delay (s/veh)										0.1																				
Approach LOS																														

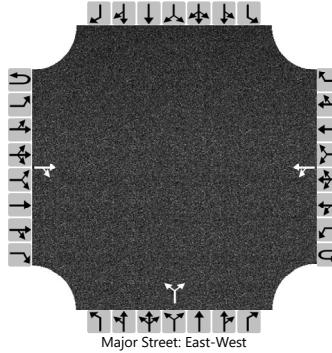
HCS7 Two-Way Stop-Control Report

General Information				Site Information																																
Analyst	TM			Intersection			Techny at W School																													
Agency/Co.	GHA			Jurisdiction			CCDOH																													
Date Performed	12/12/2019			East/West Street			Techny Road																													
Analysis Year	2019			North/South Street			West School Access																													
Time Analyzed	7:30-8:30 AM			Peak Hour Factor			0.92																													
Intersection Orientation	East-West			Analysis Time Period (hrs)			0.25																													
Project Description	Existing AM																																			
Lanes																																				
 Major Street: East-West																																				
Vehicle Volumes and Adjustments																																				
Approach	Eastbound				Westbound				Northbound				Southbound																							
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U																							
Priority	1U	1	2	3	4U	4	5	6	7	8	9	10	11																							
Number of Lanes	0	0	1	0	0	0	1	0	0	1	0	0	0																							
Configuration	TR			LT			LR																													
Volume (veh/h)	555			15			334			3			5																							
Percent Heavy Vehicles (%)				0						0																										
Proportion Time Blocked																																				
Percent Grade (%)							0																													
Right Turn Channelized																																				
Median Type Storage	Undivided																																			
Critical and Follow-up Headways																																				
Base Critical Headway (sec)							4.1			7.1			6.2																							
Critical Headway (sec)							4.10			6.40			6.20																							
Base Follow-Up Headway (sec)							2.2			3.5			3.3																							
Follow-Up Headway (sec)							2.20			3.50			3.30																							
Delay, Queue Length, and Level of Service																																				
Flow Rate, v (veh/h)							16			9																										
Capacity, c (veh/h)							928			360																										
v/c Ratio							0.02			0.02																										
95% Queue Length, Q ₉₅ (veh)							0.1			0.1																										
Control Delay (s/veh)							8.9			15.2																										
Level of Service (LOS)							A			C																										
Approach Delay (s/veh)							0.6			15.2																										
Approach LOS										C																										

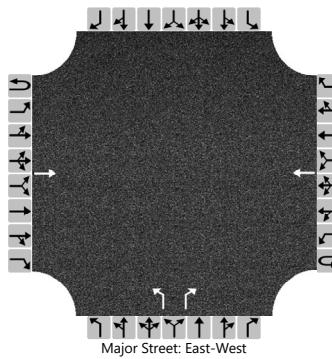
HCS7 Two-Way Stop-Control Report

General Information				Site Information																										
Analyst	TM			Intersection		Techny at W School																								
Agency/Co.	GHA			Jurisdiction		CCDOH																								
Date Performed	12/12/2019			East/West Street		Techny Road																								
Analysis Year	2019			North/South Street		West School Access																								
Time Analyzed	3:30-4:30 PM			Peak Hour Factor		0.97																								
Intersection Orientation	East-West			Analysis Time Period (hrs)		0.25																								
Project Description	Existing PM School																													
Lanes																														
 Major Street: East-West																														
Vehicle Volumes and Adjustments																														
Approach	Eastbound			Westbound			Northbound			Southbound																				
Movement	U	L	T	R	U	L	T	R	U	L	T	R																		
Priority	1U	1	2	3	4U	4	5	6	7	8	9	10																		
Number of Lanes	0	0	1	0	0	0	1	0	0	1	0	0																		
Configuration	TR			LT			LR																							
Volume (veh/h)	298			8			600			6																				
Percent Heavy Vehicles (%)				0						0																				
Proportion Time Blocked																														
Percent Grade (%)							0																							
Right Turn Channelized																														
Median Type Storage	Undivided																													
Critical and Follow-up Headways																														
Base Critical Headway (sec)				4.1			7.1			6.2																				
Critical Headway (sec)				4.10			6.40			6.20																				
Base Follow-Up Headway (sec)				2.2			3.5			3.3																				
Follow-Up Headway (sec)				2.20			3.50			3.30																				
Delay, Queue Length, and Level of Service																														
Flow Rate, v (veh/h)				8			10																							
Capacity, c (veh/h)				1239			378																							
v/c Ratio				0.01			0.03																							
95% Queue Length, Q ₉₅ (veh)				0.0			0.1																							
Control Delay (s/veh)				7.9			14.8																							
Level of Service (LOS)				A			B																							
Approach Delay (s/veh)				0.2			14.8																							
Approach LOS							B																							

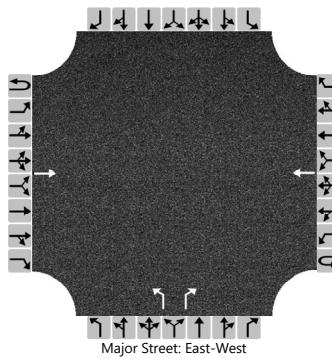
HCS7 Two-Way Stop-Control Report

General Information				Site Information																										
Analyst	TM			Intersection		Techny at W School																								
Agency/Co.	GHA			Jurisdiction		CCDOH																								
Date Performed	12/12/2019			East/West Street		Techny Road																								
Analysis Year	2019			North/South Street		West School Access																								
Time Analyzed	5:00-6:00 PM			Peak Hour Factor		0.97																								
Intersection Orientation	East-West			Analysis Time Period (hrs)		0.25																								
Project Description	Existing PM Street																													
Lanes																														
 Major Street: East-West																														
Vehicle Volumes and Adjustments																														
Approach	Eastbound			Westbound			Northbound			Southbound																				
Movement	U	L	T	R	U	L	T	R	U	L	T	R																		
Priority	1U	1	2	3	4U	4	5	6	7	8	9	10	11	12																
Number of Lanes	0	0	1	0	0	0	1	0	0	1	0	0	0	0																
Configuration	TR			LT			LR																							
Volume (veh/h)	354			6			684			16			5																	
Percent Heavy Vehicles (%)				0			0			0																				
Proportion Time Blocked																														
Percent Grade (%)							0																							
Right Turn Channelized																														
Median Type Storage	Undivided																													
Critical and Follow-up Headways																														
Base Critical Headway (sec)				4.1			7.1			6.2																				
Critical Headway (sec)				4.10			6.40			6.20																				
Base Follow-Up Headway (sec)				2.2			3.5			3.3																				
Follow-Up Headway (sec)				2.20			3.50			3.30																				
Delay, Queue Length, and Level of Service																														
Flow Rate, v (veh/h)				6			22																							
Capacity, c (veh/h)				1193			282																							
v/c Ratio				0.01			0.08																							
95% Queue Length, Q ₉₅ (veh)				0.0			0.2																							
Control Delay (s/veh)				8.0			18.8																							
Level of Service (LOS)				A			C																							
Approach Delay (s/veh)				0.1			18.8																							
Approach LOS							C																							

HCS7 Two-Way Stop-Control Report

General Information				Site Information																										
Analyst	TM			Intersection		Techny at E School																								
Agency/Co.	GHA			Jurisdiction		CCDOH																								
Date Performed	12/12/2019			East/West Street		Techny Road																								
Analysis Year	2019			North/South Street		East School Access																								
Time Analyzed	7:30-8:30 AM			Peak Hour Factor		0.93																								
Intersection Orientation	East-West			Analysis Time Period (hrs)		0.25																								
Project Description	Existing AM																													
Lanes																														
 Major Street: East-West																														
Vehicle Volumes and Adjustments																														
Approach	Eastbound			Westbound			Northbound			Southbound																				
Movement	U	L	T	U	L	T	U	L	T	U	L	T																		
Priority	1U	1	2	3	4U	4	5	6	7	8	9	10																		
Number of Lanes	0	0	1	0	0	0	1	0	1	0	0	0																		
Configuration			T			T		L		R																				
Volume (veh/h)			560			328		21		9																				
Percent Heavy Vehicles (%)								0		0																				
Proportion Time Blocked																														
Percent Grade (%)								0																						
Right Turn Channelized								No																						
Median Type Storage	Undivided																													
Critical and Follow-up Headways																														
Base Critical Headway (sec)								7.1		6.2																				
Critical Headway (sec)								6.40		6.20																				
Base Follow-Up Headway (sec)								3.5		3.3																				
Follow-Up Headway (sec)								3.50		3.30																				
Delay, Queue Length, and Level of Service																														
Flow Rate, v (veh/h)								23		10																				
Capacity, c (veh/h)								289		503																				
v/c Ratio								0.08		0.02																				
95% Queue Length, Q ₉₅ (veh)								0.3		0.1																				
Control Delay (s/veh)								18.5		12.3																				
Level of Service (LOS)								C		B																				
Approach Delay (s/veh)							16.6																							
Approach LOS							C																							

HCS7 Two-Way Stop-Control Report

General Information				Site Information																										
Analyst	TM			Intersection		Techny at E School																								
Agency/Co.	GHA			Jurisdiction		CCDOH																								
Date Performed	12/12/2019			East/West Street		Techny Road																								
Analysis Year	2019			North/South Street		East School Access																								
Time Analyzed	3:30-4:30 PM			Peak Hour Factor		0.95																								
Intersection Orientation	East-West			Analysis Time Period (hrs)		0.25																								
Project Description	Existing PM School																													
Lanes																														
 Major Street: East-West																														
Vehicle Volumes and Adjustments																														
Approach	Eastbound			Westbound			Northbound			Southbound																				
Movement	U	L	T	U	L	T	U	L	T	U	L	T																		
Priority	1U	1	2	4U	4	5	6	7	8	9	10	11																		
Number of Lanes	0	0	1	0	0	0	1	0	1	0	0	0																		
Configuration			T			T		L		R																				
Volume (veh/h)			302			578		30		12																				
Percent Heavy Vehicles (%)								0		0																				
Proportion Time Blocked																														
Percent Grade (%)								0																						
Right Turn Channelized								No																						
Median Type Storage	Undivided																													
Critical and Follow-up Headways																														
Base Critical Headway (sec)								7.1		6.2																				
Critical Headway (sec)								6.40		6.20																				
Base Follow-Up Headway (sec)								3.5		3.3																				
Follow-Up Headway (sec)								3.50		3.30																				
Delay, Queue Length, and Level of Service																														
Flow Rate, v (veh/h)								32		13																				
Capacity, c (veh/h)								297		719																				
v/c Ratio								0.11		0.02																				
95% Queue Length, Q ₉₅ (veh)								0.4		0.1																				
Control Delay (s/veh)								18.6		10.1																				
Level of Service (LOS)								C		B																				
Approach Delay (s/veh)							16.1																							
Approach LOS							C																							

Pedestrian Level of Service

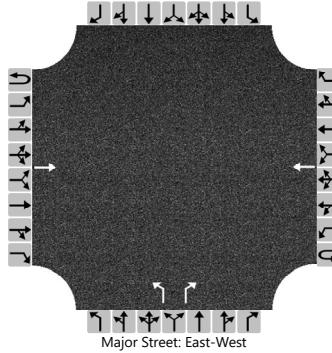
Flow (ped/hr)	0	0	5	
Two-Stage Crossing			No	
Pedestrian Platooning			No	
Conflicting Vehicular Flow (veh/h)				
Average Delay (s)			0.2	
Level of Service (LOS)			A	

Copyright © 2019 University of Florida. All Rights Reserved.

HCS™ TWSC Version 7.8.5
Existing PM School Techny at E School.xtw

Generated: 12/12/2019 11:43:18 AM

HCS7 Two-Way Stop-Control Report

General Information				Site Information																										
Analyst	TM			Intersection		Techny at E School																								
Agency/Co.	GHA			Jurisdiction		CCDOH																								
Date Performed	12/12/2019			East/West Street		Techny Road																								
Analysis Year	2019			North/South Street		East School Access																								
Time Analyzed	5:00-6:00 PM			Peak Hour Factor		0.95																								
Intersection Orientation	East-West			Analysis Time Period (hrs)		0.25																								
Project Description	Existing PM Street																													
Lanes																														
 Major Street: East-West																														
Vehicle Volumes and Adjustments																														
Approach	Eastbound			Westbound			Northbound			Southbound																				
Movement	U	L	T	U	L	T	U	L	T	U	L	T																		
Priority	1U	1	2	3	4U	4	5	6	7	8	9	10																		
Number of Lanes	0	0	1	0	0	0	1	0	1	0	0	0																		
Configuration			T			T		L		R																				
Volume (veh/h)			359			680		10		10																				
Percent Heavy Vehicles (%)								0		0																				
Proportion Time Blocked																														
Percent Grade (%)								0																						
Right Turn Channelized								No																						
Median Type Storage	Undivided																													
Critical and Follow-up Headways																														
Base Critical Headway (sec)								7.1		6.2																				
Critical Headway (sec)								6.40		6.20																				
Base Follow-Up Headway (sec)								3.5		3.3																				
Follow-Up Headway (sec)								3.50		3.30																				
Delay, Queue Length, and Level of Service																														
Flow Rate, v (veh/h)								11		11																				
Capacity, c (veh/h)								239		673																				
v/c Ratio								0.04		0.02																				
95% Queue Length, Q ₉₅ (veh)								0.1		0.0																				
Control Delay (s/veh)								20.8		10.4																				
Level of Service (LOS)								C		B																				
Approach Delay (s/veh)							15.6																							
Approach LOS							C																							